

# Stress Indices for Straight Pipe with Trunnion Attachments



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Technical Report

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## **Stress Indices for Straight Pipe with Trunnion Attachments**

TR-110162

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Prepared for **EPRI** 3412 Hillview Avenue Palo Alto, California 94304

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### **REPORT SUMMARY**

This report provides equations based on experimental and test data for determining B and C stress indices and the flexibility factor, k, for straight pipe with trunnions (or hollow circular cross-section welded attachments). The report contains explicit modifications to ASME Code Cases 391 and 392 for qualification of trunnions on pipe. It also provides flexibility equations for a more accurate evaluation of these configurations.

### Background

Fatigue is a significant consideration in the design and engineering of piping systems. The ASME Section III Code uses factors such as  $C_2$  and  $K_2$  indices to account for fatigue effects produced by reversing loads and the k flexibility factors for evaluation of piping configurations. ASME Code Cases 391 and 392 provide procedures for evaluating the design of trunnion attachments on Classes 1, 2, and 3 pipe.

### Objectives

- To derive expressions for B<sub>2</sub>, C<sub>2</sub>, K<sub>2</sub>, and k factors for trunnions on straight pipe.
- To provide modifications to Code Cases 391 and 392 for improved evaluation of trunnions on straight pipe.

### Approach

A review of the present approach for evaluation of trunnions on pipe in accordance with the Code provided an understanding of the conservatism in determining the fatigue factors. Available data on studies, experiments, and testing were collected and reviewed. Tests and analyses were performed on representative models, and the results were compared to existing data. The present values of  $A_o$ , B, and C in Code Cases 391 and 392 were modified as a result of this research and analysis. Equations and parameter limitations were derived for the determination of flexibility factors.

### Results

The report summarizes: the available literature in Section 2; the test program in Section 3; the analysis of other test data in Section 4; the comparison to Code Case results in Section 5; the finite element analysis (FEA) investigation of flexibility in Section 6; and the results of the investigation of straight pipe with trunnion attachments in Section 7.

### **EPRI** Perspective

Design for fatigue is a major concern for any power or process facility. Accurate methods of engineering for fatigue are important for cost-effective design, root cause failures, and the evaluation of remaining fatigue life of plant designs. The work being conducted under EPRI's stress intensification factor (SIF) optimization program continues to establish the technical justification to all for reductions in current Code stress indices. These reductions and associated reductions in design stresses can provide a basis to reduce the scope of on-going pressure boundary component testing and inspection programs for operating nuclear power plants. Examples include reductions in both the inspection scope of postulated high- and moderate-energy line break locations and snubber testing.

### TR-110162

### **Interest Categories**

Piping, reactor vessels, and internals

## ABSTRACT

This report was prepared under the auspices of the EPRI project on stress intensification factor (SIF) optimization. SIFs and stress indices are used in the qualification of piping components to ensure that they have an adequate fatigue life under cyclic loading. They are also used for qualification of other loading conditions. In some cases, such as trunnions, stress indices are used in lieu of SIFs.

Generally, trunnions are used on straight pipe as supports; however, they are also used as anchors. The qualification of trunnions is a major concern in the design and qualification of many piping systems. This report presents the results of an investigation of the stress indices and flexibility factors for trunnions on straight pipe subject to bending and twisting moments. The report also reviews existing data and methodologies used for qualification of trunnions. Modified expressions for stress indices are defined, and the results of new testing are included. Finally, flexibility factors are presented for accurately modeling the behavior of a trunnion in a piping system. The information presented in this report should significantly improve the qualification of trunnions on straight pipe.

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# **1** INTRODUCTION

This report was prepared under the auspices of the EPRI project RP-3921 on stress intensification factor (SIF) optimization. SIFs are used to ensure that piping has an adequate fatigue life under cyclic loading. SIFs are not used explicitly for design of trunnion/pipe configurations; however, the approach is the same.

This report specifically investigates the fatigue behavior of trunnions welded on straight pipe with full penetration welds. Trunnions are used on pipe as pipe supports. They are also referred to as "hollow circular cross-section attachments."

Trunnions on straight pipe are very similar to unreinforced branch connections. The difference is that there is no opening in the "run" pipe. As such, they are among the most complex of piping components for evaluation. Stress concentration occurs at or near the intersection of the trunnion and pipe similar to branch connections.

The general approach followed in this report is:

- 1. Review the present approach used for evaluation in accordance with the ASME Code.
- 2. Perform a literature search on the applicable references.
- 3. Perform tests, as required, and analyze the results.
- 4. Develop an updated approach to evaluate the trunnion/pipe configuration using the test data and analysis.

Section 2 of this report summarizes the present Code approaches to addressing trunnions on pipe [1,2]. The approach for Section III is in terms of two Code Cases; one for Class 1 piping, and one for Classes 2 and 3 piping [1,3,4]. The background of the Code Cases is provided. Other references are also discussed.

Section 3 of this report presents the results of fatigue tests on trunnions on straight pipe, conducted under the auspices of the EPRI research project. The test results are used to derive experimentally based expressions for the various indices. Section 4 provides an analysis of test data for use in determining B indices. Section 5 provides a comparison

#### Introduction

of test data to evaluation methods as presented in Code Case N-391-2 [3]. The flexibility of this assembly is covered in Section 6.

Section 7 of this report summarizes the conclusions of the research effort. These conclusions provide new understanding of the behavior of trunnions and allows the user to more accurately evaluate trunnions on straight pipe.

# 2 BACKGROUND

### Nomenclature

Figure 2-1 shows the configuration and applied moments for evaluation of stress indices for straight pipe with trunnion attachments. The nomenclature includes terminology used in the body of this report and the associated appendices.



Figure 2-1 Trunnion/Pipe Connection

 $R_{o}$  = run pipe outside radius, inch (in.)  $r_0 =$  trunnion outside radius, in.  $r_i$  = trunnion inside radius, in. T = nominal run pipe wall thickness, in.t = nominal trunnion wall thickness, in.  $D = D_{o} - T$  $\mathbf{d} = \mathbf{d}_{o} - \mathbf{t}$  $D_{o}$  = outside diameter of the run pipe, in.  $d_{0}$  = outside diameter of the trunnion, in.  $R_m$  = mean radius of run pipe, in.  $A_{T} = \pi (r_{0}^{2} - r_{i}^{2})$  $Z_T = I_T / r_o$  $I_{p} = \pi (R_{o}^{4} - R_{i}^{4})/4$  $I_{T} = \pi/4(r_{0}^{4} - r_{1}^{4})$  $A_{m} = \pi/2 (r_{0}^{2} - r_{1}^{2})$  $J_{m} = lesser of \pi r_{o}^{2}T or Z_{T}$  $\delta$  = test displacement amplitude

Background

 $\gamma = R_{o}/T$  $\varphi_{\scriptscriptstyle ii}$  = rotation at point i with respect to point j $\phi_x, \phi_y, \phi_z$  = rotations about the x, y, or z axis  $\tau = t/T$  $\beta = d_0 / D_0$  $C = A_{0} (2\gamma)^{n1} \beta^{n2} \tau^{n3}$  but not less than 1.0 k = test specimen stiffness  $F = test k * test \delta$ L = length from test load point to outside diameter (OD) of specimen (in.) N = test cycles to failure  $M = F^*L$  $Ma = 10000^{*}[.7854^{*}(d/2)^{2*}t]$  $Mb = 10000^{*}[.7854^{*}(D/2)^{2*}T]$  $M_{c}$  = range of resultant moments due to thermal expansion, inch-pounds (in.-lbs.)  $M_{L}$  = bending moment applied to the trunnion, as shown in Figure 2-1, in.-lbs.  $M_{N}$  = bending moment applied to the trunnion, as shown in Figure 2-1, in.-lbs.  $M_{T}$  = torsional moment applied to the trunnion, as shown in Figure 2-1, in.-lbs.  $Q_1$  = shear load applied to the trunnion, as shown in Figure 2-1, pound (lb.)  $Q_2$  = shear load applied to the trunnion, as shown in Figure 2-1, lb. W = thrust load applied to the trunnion, as shown in Figure 2-1, lb. [These moments and loads are determined at the surface of the pipe.] n<sub>1</sub>, n<sub>2</sub>, n<sub>3</sub> are specified in Code Cases N-391 or N-392 (See Appendix A or B.)  $C_{\scriptscriptstyle\rm T}$  = 1.0 for  $\beta \leq 0.55$  $C_T = C_N$  for  $\beta = 1.0$ , but not less than 1.0;  $C_T$  should be linearly interpolated for  $0.55 < \beta < 1.0$  $C_1$  = values of  $C_1$  based on fatigue test data  $B_w = 0.5(C_w)$ , but not less than 1.0  $B_1 = 0.5(C_1)$ , but not less than 1.0  $B_{N} = 0.5(C_{N})$ , but not less than 1.0  $B_{T} = 0.5(C_{T})$ , but not less than 1.0  $B_{T}' =$  values of  $B_{T}$  based on limit load test data K<sub>e</sub> = plasticity factor used in fatigue analysis  $K_{T} = 1.8$  for full penetration welds  $T_{T}$  = average temperature of that portion of the trunnion within a distance of 2t from the surface of the pipe, degrees Fahrenheit (°F)  $T_{w}$  = average temperature of that portion of the pipe under the attachment and within a distance of (RT)<sup>0.5</sup> from the edge of the attachment, °F  $E\alpha$  = modulus of elasticity, E, times the mean coefficient of thermal expansion,  $\alpha$ , both at room temperature, pounds per square inch (psi)/°F S<sub>alt</sub> = stress amplitude (psi)  $W^{"}$ ,  $M_{N}^{"}$ ,  $M_{L}^{"}$ ,  $Q_{1}^{"}$ ,  $Q_{2}^{"}$ , and  $M_{T}^{"}$  are absolute values of maximum loads occurring simultaneously under all service loading conditions

### ASME Section III and B31.1 Power Piping Code Approach

The body of the present Codes, Section III and ANSI B31.1, are silent in regards to specific methodologies for qualification of trunnion/pipe configurations [1,2]. However, Section III has two Code Cases (N-391 for Class 1 piping and N-392 for Classes 2 and 3 piping) that address the evaluation of the design of trunnion attachments on straight pipe [3,4]. These Code Cases are included in Appendix A and Appendix B for reference. For simplicity, this report will refer to both of these Code Cases as the "Code Case" when discussing common items.

N-391 requires the calculation of various stresses:

$$S_{MT} = B_{W}W/A_{T} + B_{N}M_{N}/Z_{T} + B_{L}M_{L}/Z_{T} + Q_{1}/A_{m} + Q_{2}/A_{m} + B_{T}M_{T}/J_{m}$$
(Eq. 2-1)  

$$S_{NT} = C_{W}W/A_{T} + C_{N}M_{N}/Z_{T} + C_{L}M_{L}/Z_{T} + Q_{1}/A_{m} + Q_{2}/A_{m} + C_{T}M_{N}/Z_{T} + C_{T}M_{T}J_{m} + 1.7E\alpha |T_{T}-T_{W}|$$
(Eq. 2-2)

$$S_{PT} = K_T (S_{NT})$$
 (Eq. 2-3)

$$S_{NT}^{"} = C_{W}W^{"}/A_{T} + C_{N}M_{N}^{"}/Z_{T} + C_{L}M_{L}^{"}/Z_{T}$$
$$+ Q_{1}^{"}/A_{m} + Q_{2}^{"}/A_{m} + C_{T}M_{T}^{"}/J_{m}$$
(Eq. 2-4)

N-392 has similar expressions, except that the  $1.7E\alpha |T_T - T_w|$  term in Equation 2-2 is not included. The stresses calculated by these equations are used in the qualification in modified standard Code equations by the two Code Cases. This report focuses on the B and C indices.

Rodabaugh discusses the background of N-391 and N-392 and is summarized herein [5]. It should be noted that the original objective in developing these Code Cases was to provide a simplified and conservative methodology. The approach used to address the effects of the various mechanical loads (W,  $Q_1$ ,  $Q_2$ ,  $M_N$ ,  $M_L$ , and  $M_T$ ) is discussed below.

The original basis for considering the effects of the W,  $M_L$ , and  $M_N$  loads was the correlation equations given by Potvin [6]. These correlation equations were considered to correspond to the maximum primary-plus-secondary stresses ( $P_L + P_b + Q$ ). Thus, they corresponded to the C indices of NB-3600 or  $C_W$ ,  $C_L$ , and  $C_N$  of the Code Cases [1,3,4].

### Background

The form of the Code Case expression for the C indices is:

$$C = A_{0}(2\gamma)^{n1}\beta^{n2}\tau^{n3}$$
 (Eq. 2-5)

where the parameters are defined by  $\gamma = R_o/T$ ,  $\beta = d_o/D_o$ , and  $\tau = t/T$ .  $A_o$ , n1, n2, and n3 are constants that vary depending upon the loading direction. (See Appendix A or Appendix B.)

The range of the applicable parameters in the Code Cases for  $C_w$ ,  $C_L$ , and  $C_N$  has been extended beyond that of Potvin [6]. The applicable range of  $\gamma$  and  $\tau$  was extended based on Welding Research Council (WRC) Bulletin 198 and WRC Bulletin 297 [7,8]. The range of  $\beta$  was extended based on comparison with the equations derived by Wordsworth [9].

At the time the Code Cases were prepared, data were not available regarding shear loads and torsional moments ( $Q_1$ ,  $Q_2$ , and  $M_T$ ). Engineering judgment was used in the evaluation of their effects. For the shear loads ( $Q_1$  and  $Q_2$ ), the stress intensity (twice the shear stress) is  $Q/A_m$ , where  $A_m$  is one-half the cross-sectional area of the trunnion-pipe interface (where the load is taken) assumed to be  $\pi(r_o^2 - r_i^2)/2$ . This is considered reasonable for small trunnions (small  $d_o/D_o$ ) but is probably very conservative for large trunnions (for example, size-on-size).

The approach used to evaluate the effects of  $M_T$  was based on comparisons to data on branch connections [10]. Branch connections are similar to trunnions, except that the run pipe has an opening in it. For branch connections with small  $d_o/D_o$ , the stress intensity is about  $M_t/J_m$ . For  $d_o/D_o = 1.0$ , test data indicates that the maximum stress intensity is about the same as for out-of-plane bending (for example, due to  $M_N$ ) [11]. Based on this information, the value of  $C_T$  was taken as 1.0 for  $\beta = d_o/D_o \le 0.55$  and equal to  $C_N$  for  $\beta = 1.0$ . Linear interpolation is used in between. The change at  $\beta = 0.55$  corresponds to Potvin's data.

Potvin originally suggested a limit on  $\gamma = R_{o}/T \ge 8.33$ . Rodabaugh provides a basis for extending that to  $\gamma = R_{o}/T \ge 4.0$  [5]. This was based on a comparison with Wordsworth [9]. This change was made in Code Case N-392 but not in N-391; however, this extension is valid for N-391.

The B indices that are in the Code Cases correspond to those of ASME Section III, NB-3600. The B indices are based upon limit load (or moment) analysis or test. The Code Cases take the B indices as one-half the C indices. Based upon data in Rodabaugh and Kurobane, it is stated that the Code Case B indices are conservative by "a factor of at least 1.5" [5,11,12].

The approach followed by the Code Case is to calculate the stresses due to the trunnion mechanical loads (W,  $Q_1$ ,  $Q_2$ ,  $M_N$ ,  $M_L$ , and  $M_T$ ) and the thermal stresses (if Class 1 piping) and add them to the stresses in the pipe due to loads in the pipe. The stresses are added linearly and then compared to the specific limits dependent upon the piping class and the specific requirement. The linear addition of stresses is generally very conservative. It assumes that all the stresses are maximum at the same point.

### **Review of References**

In addition to the references discussed in the preceding section, there are other references that provide additional information. Slagis, Hankinson, and Hankinson provide general discussions of the subject, including a discussion regarding jurisdictional boundaries [14,15,16].

Melworm, Sadd, Gray, and Basavaraju provide additional information regarding finite element analysis of various configurations and load applications [17,18,19,20].

As discussed earlier, the linear addition of stresses is very conservative. Gray provides such an example [19]. Using this approach, a cumulative usage factor greater than 6 was calculated. Detailed finite element analysis yielded a cumulative usage factor less than 1.0.

# **3** TEST PROGRAM

### Purpose

The purpose of this test program was to obtain some specific data that corresponded to the test methodology followed by Markl [13]. These tests would provide data that could be used to investigate the design approach suggested by Code Cases N-391 or N-392. For simplicity, this report will refer to both of these Code Cases as the "Code Case" when discussing common items.

### Methodology

**Design Of Test Specimens** Four specimens were manufactured by Wilson Welding Service, Incorporated, of Decatur, Georgia. The test specimens consisted of 8-inch NPS Schedule 20 A53-B pipe with a 4-inch Schedule 40 A53-B trunnion. The welds at the interface of the trunnion and pipe were normal full penetration in an as-welded condition. The test specimens were labeled A, B, C, and D. Figure 3-1 indicates the test configuration.





**Testing Program** The testing was performed at The Ohio State University. The fatigue tests were performed on an MTS Systems Corporation Series 319 dynamically rated Axial/Torsional Load Frame. This unit is designed to accommodate either uniaxial or multiaxial testing. Load frame capacities are 55,000 pounds axial force and 20,000 in.-lb. torsional moment. A computerized control panel provides local, precise operations of the cross head, hydraulic grips, and actuator. The maximum actuator displacement is 6 inches. The loading pattern applied to an attached sample is controlled by programmable servovalves.

Built in loading programs include sinusoidal and triangular waves with the user being able to select, within machine limits, the desired amplitude and frequency. The actual

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displacement of the actuator is measured by a linear variable differential transformer (LVDT). The output of either the load cell or the LVDT can be selected for closed loop control of the actuator displacement time history. During a test, the number of cycles of applied load is recorded by a digital counter and displayed on the MTS console.

In these tests, the load was sinusoidal at frequencies ranging from 0.3 Hertz (Hz)– 0.5 Hz. Actuator displacement was designated the test control variable. The selection of displacement as the control parameter meant that actuator movement was used by the MTS system for the feedback in the closed loop controls. This resulted in virtually identical cycles of actuator displacement being recorded throughout the duration of each test. The load resulting from the imposition of the specified displacement was measured with a fatigue-rated, 5,000 lb. capacity, tension-compression, electronic load cell manufactured by the Lebow Instrument Company. The output of this load cell was monitored continuously throughout the duration of each test.

Both load and actuator displacement were recorded using a computer program written at OSU, in LabVIEW<sup>TM1</sup>, specifically for that purpose. LabVIEW is a graphical language developed by National Instruments that allows the user to design, in software, a test control and data collection system adapted to the requirements of each experimental program. In the LabVIEW application developed for the fatigue tests, the signals from the load and displacement transducers were sampled 30 times per second, and the time histories of each were plotted on the computer screen in real time so that the progress of the test could be readily monitored. By combining the load and displacement time histories, a plot of load vs. displacement at any load cycle desired could be constructed. This, too, was done in real time so that changes in the response of the test specimen could be identified while the specimen was still undergoing loading. Any of these presentations of the test data could be printed while the test was still in progress.

Figure 3-1 shows the load application point and direction of loading. Note that the distance from the load point to the surface of the pipe (~46 inches) varies slightly for each test specimen. The measured distance (L) which is dependent on the installation is included in the test data.

The test data, results and other information are provided in Appendix C. The tests were displacement-controlled cantilever bending tests. The tests followed the standard approach corresponding to Markl type tests [13,21]. Each specimen was first tested to determine the load deflection curve for that particular specimen. The load deflection curve was used to determine the stiffness of each specimen and the load applied to the specimen by a given amount of displacement. The load deflection curves were determined for loading in both positive and negative loading directions (down and

<sup>&</sup>lt;sup>1</sup> LabVIEW is a trademark of National Instruments Corporation.

up). Each specimen was then fatigue-tested by cycling the deflection in both directions of loading by a controlled amount. The cycles to failure were counted to determine the fatigue life. Failure was detected when through-wall cracks formed and water leaked though the cracks.

Test Results Summary Table 3-1 provides a summary of the test results.

Table 3-1Summary of Test Results

TEST	δ	k	<b>F</b> <sup>1</sup>	L²	Z <sub>τ</sub>	М	Ν	i,³
	in.	lbs./in.	lbs.	in.	in. <sup>3</sup>	inlbs.	Cycles to Failure	
A	1.05	1741	1828	46.0625	3.21	84099	500	2.700
В	0.90	1732	1559	45.50	3.21	70935	1,280	2.656
С	0.90	1702	1532	46.125	3.21	70664	934	2.839
D	0.80	1841	1473	45.50	3.21	67022	1,866	2.606

Notes: (1)  $F = \delta * k$ .

(2) Refer to Figure 3-1 for location of L.

(3) The value of  $i_t$  is calculated from  $i_t = 245,000 \text{ N}^{-0.2}/\text{S}$ , where N = cycles to failure and S = M/Z<sub>T</sub>. Z<sub>T</sub> is based on nominal dimensions for the trunnion.

### **Analysis of Test Data**

There are several methods available to analyze the data. In general, for this type of loading condition, the purpose of analysis is to be able to express the results in terms of SIFs (or i-factors),  $B_2$  indices,  $C_2$  indices, and/or  $K_2$  indices. As the applicable Code Cases do not use SIFs in the qualification of trunnion/pipe configurations, the focus will be on  $B_2$ ,  $C_2$ , and  $K_2$ . Because the welds were full penetration welds, it is believed that the Code Case specification that  $K_2 = 1.8$  is reasonable. Hence, the focus will be on  $B_2$  and  $C_2$ .  $C_2$  will be covered first. (Note that the Code uses the terms  $B_2$ ,  $C_2$ , and  $K_2$ , and the Code Case uses subscripts that indicate direction of loading, etc.)

### C Indices—Markl Approach

As discussed earlier, the tests that were performed as a part of this investigation were fatigue tests. There are two methods that can be used to evaluate the results. The first

will be referred to as the Markl approach. The second is the Class 1 fatigue approach used in Class 1 analysis per NB-3600 [1]. The Markl approach will be investigated first.

The fatigue tests on the trunnion/pipe configurations followed the Markl approach [13,21]. Markl used the following expression for Grade B carbon steel:

 $iS = 245,000 N^{-0.2}$  (Eq. 3-1)

where S is the nominal stress in the component and N is the number of cycles when through-wall cracks occur and water leaks. This is used as the definition of the SIF (i factor) and is used in the design for fatigue for B31.1 piping and ASME Section III Classes 2 and 3 piping [1,2].

The Code cases differ in that they use C indices (and other indices) in the evaluation of fatigue instead of SIFs [3,4]. The C indices correspond to primary-plus-secondary stresses. The  $C_2$  indices, which are applicable to moment loading in piping, are related to the SIFs. Section NC-3672.2 provides the following equation:

$$i = C_2 K_2 / 2$$
 (Eq. 3-2)

This expression will be used to evaluate the value of  $C_2$ , which is used in the Code Cases [1]. The approach follows that developed in Rawls [22].

In N-392, the following equation is provided:

$$S_{E} = iM_{c}/Z + S_{PT}/2$$
 (Eq. 3-3)

and also:

$$S_{\rm PT} = K_{\rm T} S_{\rm NT} \tag{Eq. 3-4}$$

For this application, (neglecting the shear stress term,  $Q_2/A$ , which is about 1 percent (%) of the bending stress):

$$S_{\rm NT} = C_{\rm L} M_{\rm L} / Z_{\rm T} \tag{Eq. 3-5}$$

Therefore:

$$S_{PT} = K_T C_L M_L / Z_T$$
 (Eq. 3-6)

and:

$$S_{\rm E} = iM_{\rm c}/Z + K_{\rm T} C_{\rm L} M_{\rm L}/(2Z_{\rm T})$$
(Eq. 3-7)

3-5

Substituting Equation 3-2 into Equation 3-7 with  $K_2 = K_T$  yields:

$$S_{\rm F} = (K_{\rm T}/2) C_{2}M/Z + (K_{\rm T}/2) C_{1}M_{1}/Z_{\rm T}$$
(Eq. 3-8)

 $S_{E}$  in Equation 3-8 is equivalent to the iS term in Equation 3-1. Substituting and rearranging yields:

$$C_{L} = \{245,000 \text{ N}^{-0.2}(2/K_{T}) - C_{2} \text{ M/Z} \} Z_{T}/M$$
 (Eq. 3-9)

As this  $C_L$  is derived from fatigue tests, to distinguish it from the  $C_L$  from the Code Case, it now will be called  $C_L'$ . Additionally, because  $C_2 = 1.0$  for straight pipe, Equation 3-9 becomes:

$$C_{L} = \{245,000 \text{ N}^{-0.2}(2/K_{T}) - M/Z\} Z_{T}/M$$
 (Eq. 3-10)

 $C_L$  is a fatigue-based value that can be compared to the value of  $C_L$  calculated from the Code Case. Table 3-2 provides this comparison.

Table 3-2 Comparison of CL to CL'

TEST	Ζ <sub>τ</sub>	z	М	Ν	<b>C</b> <sup>1</sup>	<b>K</b> <sub>2</sub> <sup>2</sup>	<b>C</b> _' <sup>3</sup>	C_/C_'
	in.³	in. <sup>3</sup>	inlbs.	Cycles to Failure				
Α	3.21	13.39	84099	500	3.77	1.8	2.76	1.37
В	3.21	13.39	70935	1,280	3.77	1.8	2.71	1.39
С	3.21	13.39	70664	934	3.77	1.8	2.91	1.30
D	3.21	13.39	67022	1,866	3.77	1.8	2.65	1.42
							Average =	1.37

Notes: 1. From CC N-392,  $C_{L} = 3.77$  for the pipe, 3.12 for the attachment, where the parameters for  $C_{L}$  are Ro/T=7.25,  $d_{0}/D_{o}$ =.522, and t/T=0.948.

2.  $K_2 = 1.8$  for full penetration welds.

3. Calculated using Equation 3-10.

Based on this evaluation, it is apparent that the value of  $C_L$  calculated in accordance with the Code Case is conservative by about 30%.

### C Indices—Class 1 Approach

The second method of evaluating the data follow the fatigue evaluation approach used for Class 1 analysis (NB-3600). Table 3-3 provides a summary of a fatigue analysis of the data using the values for the various stress indices calculated in accordance with the Code Case.

# Table 3-3 Trunnion/Pipe—Class 1 CUF Evaluation Using Code Case Indices

Case	M	SNT	C2 Mi/Z	Sn	3Sm	Ke	SPT	SP	Salt	N	N	CUF
	kips	ksi	ksi	ksi	ksi		ksi	ksi	ksi	Allowable	Failure	
Α	84.099	197.5	12.6	210.1	60.0	5.00	355.6	361.9	904.6	96	500	5.20
В	70.935	166.6	10.6	177.2	60.0	4.91	299.9	305.2	748.9	142	1280	9.03
С	70.664	166.0	10.6	176.5	60.0	4.88	298.8	304.0	742.6	144	934	6.47
D	67.022	157.4	10.0	167.4	60.0	4.58	283.4	288.4	660.6	184	1866	10.16
											Average =	7.71
Notes:	1. C <sub>2</sub> (pipe) =	1.0										<b>•</b>
	2 K <sub>2</sub> (nine) =	1.0										
	2. $K_2$ (pipe) =	1.0										
	3. $R_T$ (liuii) =	1.0	-									
	4. $C_{L}(trun) =$	3.77	<b>←</b>									
	5. Z (pipe) =	13.39	in. <sup>3</sup>									
	6. Z <sub>⊤</sub> (trun) =	3.21	in. <sup>3</sup>									
	7. $S_{NT} = C_L M_T$	Z <sub>T</sub> , where	the moment	is the rang	je.							
	8. $S_n = C_2 M_i/2$	Z + S <sub>NT</sub>										
	9. S <sub>PT</sub> = K <sub>T</sub> S <sub>N</sub>											
	10. $S_P = K_2 C_2 D_2$	$/2IM_i + S_{PT}$										
	11. Calculations	s are based	on nomina	l dimensior	is.							
	12. Ke is the fra	actor for ela	stic-plastic	analysis d	efined in NE	3-3228.5, R	eference 1.					
	13. S., = S. K./	2										
	an epre											

The test cumulative usage factor, or CUF, is based on an allowable number of cycles from the expression:

$$N_{\text{allowable}} = (8,664,000/(S_{\text{alt}}-21,645))^2$$
 (Eq. 3-11) [26]

This expression does not include the factors of safety of 2 on stress and 20 on cycles that are part of the Section III Class 1, Appendix I, S-N design curves [1]. If these were included, the calculated CUF would be much greater.

The value of  $S_m$  used was 20 kips per square inch (ksi) as specified by the Code. As indicated in Table 3-3, the CUF for the tests is, on the average, 7.99 versus a Code requirement of 1.0. This indicates that the Code is very conservative. Contributors to the conservatism could be the value of the indices and/or the value of  $K_e$ . If the Code approach to  $K_e$  is changed in the future, the values of  $C_L$  based on the Class 1 would need to be reviewed.

Table 3-4 presents the results of a fatigue analysis in which the value of  $C_L$  was varied until the average CUF = 1.00. The corresponding value of  $C_L$  was 2.36. The ratio of  $C_L$ 

(Code Case)/ $C_L$ (Test) = 3.77/2.36 = 1.597, which indicates the Code Case is conservative by about 60%.

#### Table 3-4 Trunnion/Pipe—Class 1 CUF Evaluation, Average CUF = 1.0

Case	M	S <sub>NT</sub>	C <sub>2</sub> M <sub>i</sub> /Z	Sn	3S <sub>m</sub>	Ke	S <sub>PT</sub>	SP	Salt	N	N	CUF
	kips	ksi	ksi	ksi	ksi		ksi	ksi	ksi	Allowable	Failure	
Α	84.099	123.7	12.6	136.2	60.0	3.54	222.6	228.9	405.2	510	500	0.98
В	70.935	104.3	10.6	114.9	60.0	2.83	187.7	193.0	273.2	1186	1280	1.08
С	70.664	103.9	10.6	114.5	60.0	2.82	187.0	192.3	270.7	1209	934	0.77
D	67.022	98.5	10.0	108.6	60.0	2.62	177.4	182.4	238.8	1590	1866	1.17
											Average =	1.00
												<b>↑</b>
Notes:	1. C <sub>2</sub> (pipe) =	1.0										
	2. K <sub>2</sub> (pipe) =	1.0										
	3. K <sub>T</sub> (trun) =	1.8										
	4. C <sub>L</sub> (trun) =	2.36	<del>(</del>									
	5. Z (pipe) =	13.39	in. <sup>3</sup>									
	6. Z <sub>T</sub> (trun) =	3.21	in. <sup>3</sup>									
	7. $S_{NT} = C_L M_T /$	Z <sub>T</sub> , where t	he moment	is the rang	je.							
	8. $S_n = C_2 M_i/Z$	2 + S <sub>NT</sub>										
	9. S <sub>PT</sub> = K <sub>T</sub> S <sub>N</sub>	т										
	10. $S_P = K_2 C_2 D_0 /$	2IM <sub>i</sub> + S <sub>PT</sub>										
	11. Calculations	are based	on nomina	dimension	IS.							
	12. Ke is the fra-	ctor for ela	stic-plastic	analysis de	fined in NB	-3228.5, Re	eference 1.					
	13. $S_{alt} = S_p K_e/2$	2										
	at p c											

In this case, the value of  $K_{e}$  varies from 2.62 to 3.54, which reduces the potential of contribution to the overall conservatism.

### **B Indices—From Test Data**

The Code Cases specify that the value of the Bindices be taken as one-half the value of the C indices, but not less than 1.0. It is worthwhile to determine if the data obtained in this study can be used for evaluating these indices.

The ASME Code uses limits on the primary stress intensity to limit gross plastic deformation of piping [1]. The Code has specific limits that it applies to stresses calculated using B-indices. The basic equations of the Code are modified to include the effects of the trunnions in the Code case. (Refer to Equation 2-1; the terms in Equation 2-1 can be neglected except for the term with B<sub>L</sub> because of the loading.)

Therefore the equation reduces to:

 $S_{MT} = B_L M_L / Z_L$  (Eq. 3-12)

Using  $S_y$  as the allowable stress and solving for  $M_L$  (the limit moment) yields:

$$\mathbf{M}_{\mathrm{L}} = \mathbf{S}_{\mathrm{Y}} \mathbf{Z}_{\mathrm{T}} / \mathbf{B}_{\mathrm{L}}$$
(Eq. 3-13)

Or rearranging:

$$B_{L} = S_{Y}Z_{T} / M_{L}$$
 (Eq. 3-14)

As this value of  $B_L$  is based on test data, it now will be referred to as  $B_L'$  to distinguish it from the value of  $B_L$  calculated from the Code case. Hence:

 $B_{L} = S_{y}Z_{T} / M_{L}$  (Eq. 3-15)

To determine the limit moment (or limit load) experimentally, a load-deflection curve must be developed. The limit moment is defined as when the deflection is equal to twice that predicted, assuming linear behavior. This is explained in Article II-1000, Section II-1430 of the ASME Boiler and Pressure Vessel Code, and it is shown in Figure 3-2 [1].



Deflection -----

Figure 3-2 Limit Load Definition

The tests performed for this report were directed toward obtaining fatigue data rather than limit moment data. However, the data that was taken during the initial phase of the testing can be used to obtain an estimate of the limit moments.

The first phase of the tests involved determining the stiffness of the test specimen. That was determined by obtaining a load-deflection curve. (See Appendix C for curves.) The loads in these tests were taken slightly into the plastic region. As such, the maximum loads can be used to estimate the limit moment. This would be a lower limit because the deflection was not allowed to go to twice that based on elastic behavior. This maximum load will be used to investigate the value of  $B_L$ '.

A review of the curves in Appendix C shows that the specimens were loaded in both the positive and negative direction. Thus  $B_L'$  can be estimated for both directions of loading.

Table 3-5 shows the calculation of  $B_L'$  based on the maximum force used in determining the load deflection curve. As noted earlier, this force is less than the limit moment; hence, it will underpredict  $B_L'$ . Column 5 lists the values of  $B_L'$  predicted.

	COLUMN			(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)
TEST	LOADING	m	L	Fmax	M=F*L	Z	Sy	$B_L = S_y Z/M$	$B_L = C_L/2$	(6)/(5)	F <sub>LM</sub> -EST	$B_L = S_y Z/M$	(6)/(9)
SPECIMEN	DIRECTION	lb./in.	in.	lbs.	inlb.	in. <sup>3</sup>	ksi		(N-392)			Using (8)	
											1		
Α	POSITIVE	1836	46.0625	2469	113,728	3.215	63.3	1.79	1.89	1.05	2850	1.55	1.22
	NEGATIVE	1566	46.0625	2650	122,066	3.215	63.3	1.67	1.89	1.13	2750	1.61	1.17
					Ave	rage for spe	cimen A =	1.73		1.09		1.58	1.19
В	POSITIVE	1797	45.5000	3224	146,692	3.215	63.3	1.39	1.89	1.36	3500	1.28	1.48
	NEGATIVE	1495	45.5000	2560	116,480	3.215	63.3	1.75	1.89	1.08	2400	1.86	1.01
					Ave	rage for spe	cimen B =	1.57		1.22		1.57	1.24
С	POSITIVE	1752	46.1250	1984	91,512	3.215	63.3	2.22	1.89	0.85	2450	1.80	1.05
	NEGATIVE	1473	46.1250	2117	97,647	3.215	63.3	2.08	1.89	0.90	2500	1.76	1.07
					Ave	rage for spe	cimen C =	2.15		0.88		1.78	1.06
D	POSITIVE	1800	45 5000	2182	00.281	3 215	63.3	2.05	1 80	0.02	2700	1.66	1 1 4
D	NEGATIVE	1090	45.5000	2102	99,201	3.213	03.3	2.03	1.09	0.92	2700	1.00	1.14
	NEGATIVE	1766	45.5000	2409	109,610	3.215	63.3	1.86	1.89	1.02	2650	1.69	1.12
					Ave	rage for spe	cimen D =	1.95		0.97		1.67	1.13
			Average	e for all spe	cimens, bot	h loading dir	rections =	1.85		1.04		1.65	1.16
										1			1

# Table 3-5 Trunnion/Pipe—Experimental Evaluation of BL'

Note that the value of  $S_{\rm y}$  is based on the material certification data provided by the test specimen manufacturer.

Column 6 lists the value of  $B_L$  calculated from the Code Case. Column 7 is the ratio of  $B_L/B_L'$ . As can be seen, this ratio is very close to 1.0. It indicates that the values of  $B_L$  (on the average) are only conservative by about 4%.

It should be noted that if  $S_y$  was based on  $S_m$  from the Code, where  $S_y = 3/2$   $S_m = 3/2$  (20) = 30 ksi., the ratio of  $B_L$  (Code)/ $B_L$ (Test) (see Column 7) would increase to 2.19.

In order to obtain more insight into the actual value of  $B_L$ , the values of the limit moments were estimated from the load-deflection curves. This was performed by extrapolating the load-deflection curves, assuming that they would continue to follow the shape of the curves beyond the point where the loading was stopped. In other words, it was assumed that there would be no sudden change in the behavior. This is believed to be a reasonable assumption.

Column 8 lists the estimated limit moments ( $F_{LIM}$ -EST). Columns 9 lists the associated value of  $B_L'$ . Column 10 lists the ratio of  $B_L/B_L'$ . It can be seen that the ratio is 1.14 on the average. If  $S_Y$  was based on  $S_m$  from the Code, the ratio of  $B_L$  (Code)/ $B_L$ (Test) (see Column 10) would increase to 2.41.

The results will be discussed in the next section.

# **4** ANALYSIS OF OTHER TEST DATA FOR B INDICES

### Introduction

The Code Case specifies that the various B indices be calculated as 0.5 times the corresponding C index. As indicated earlier, the tests performed under this program were not directed toward determination of the maximum collapse load that could lead to evaluating the B indices. An estimate was made, using available test data, that provided some insight into the values of B indices. However, there is additional test data available that could be used to investigate this area further. This is covered in this chapter.

### References

The original data to be investigated is from Gibstein, Toprac, and the *Study on Tubular Joints Used for Marine Structures* (hereafter *Tubular Joints*) and is summarized in WRC Bulletin-256 [11,23,24,25]. The data includes dimensions, yield strengths, and the "moment at maximum load." As pointed out in WRC Bulletin-256, the exact definition of maximum load is not clear for all the test data. It could correspond to rupture, cracking, deforming beyond some limit, or the maximum load corresponding to the ultimate strength of a tensile strength test.

It is likely that the definition of "maximum load" does not exactly correspond to the Code definition of "collapse load." As discussed earlier, the Code definition of collapse load is that load when the actual deflection is twice that predicted, assuming elastic behavior. Even though the definitions may be different, the data could be helpful in this investigation.

Table 4-1 lists the data from Gibstein, Toprac, and *Tubular Joints* for in-plane bending tests [23,24,25]. The dimensional data is provided. The yield strengths are also listed (for the pipe). In general, the yield strength for the trunnion is very similar. The "moment at maximum load" is listed. The column labeled  $B_L$  is calculated using Equation 3-15:

 $B_L' = S_Y Z_T / M_L$ 

Analysis of Other Test Data for B Indices

Table 4-1	
Gibstein In-Plane Bending Tests (WRC Bulletin 256-Table 2)	

D	d	т	t	Sy	Mt				B <sub>L</sub> '	CC N-392	
mm.	mm.	mm.	mm.	Kg/mm2	Kg/m	T/D	d/D	t/T	(S <sub>Y</sub> Z <sub>T</sub> /ML)	CL	B <sub>L</sub> '/C <sub>L</sub>
298.5	101.2	10.3	5.0	30.0	1420	.03	.34	0.485	0.732	2.73	0.268
298.5	108.2	10.0	6.3	30.0	2080	.03	.36	0.630	0.701	2.96	0.236
298.5	108.2	10.0	8.0	30.0	2580	.03	.36	0.800	0.684	3.25	0.211
219.1	71.6	6.3	18.5	32.0	840	.03	.33	2.937	1.298	10.22	0.127
219.1	71.6	8.9	18.5	43.0	1810	.04	.33	2.079	0.809	6.17	0.131
298.5	101.6	7.2	16.0	30.0	1460	.02	.34	2.222	1.650	8.92	0.185
219.1	101.6	5.5	16.0	31.1	1190	.03	.46	2.909	2.098	10.84	0.194
219.1	101.6	8.4	16.0	37.4	2630	.04	.46	1.905	1.142	5.84	0.195
219.1	101.6	10.0	16.0	37.5	3560	.05	.46	1.600	0.846	4.53	0.187
219.1	101.6	12.3	16.0	41.2	5500	.06	.46	1.301	0.601	3.35	0.180
219.1	139.7	6.0	17.5	32.0	2630	.03	.64	2.917	2.229	10.18	0.219
219.1	139.7	8.8	17.5	43.0	6000	.04	.64	1.989	1.313	5.82	0.226
219.1	139.7	12.3	17.5	40.0	9000	.06	.64	1.423	0.814	3.57	0.228
298.5	193.7	7.3	7.1	30.2	5450	.02	.65	0.973	1.038	4.24	0.245
298.5	193.7	10.0	7.1	30.0	8000	.03	.65	0.710	0.702	2.67	0.263
298.5	193.7	10.0	7.1	30.0	8730	.03	.65	0.710	0.644	2.67	0.241
219.1	177.8	5.9	16.0	32.0	4130	.03	.81	2.712	2.342	9.57	0.245
219.1	177.8	8.6	16.0	43.0	10000	.04	.81	1.860	1.300	5.52	0.235
219.1	177.8	12.5	16.0	40.0	16400	.06	.81	1.280	0.737	3.2	0.231
D	d d	aing ies	t t	Sv	Mt				В,'	CC N-392	
in.	in.	in.	in.	ksi	inlbs.	T/D	d/D	t/T	(S <sub>v</sub> Z <sub>⊤</sub> /ML)	C,	B <sub>1</sub> '/C <sub>1</sub>
8.66	8.66	0.279	0.279	41.2	645000	.03	1.00	1.000	0.953	3.61	0.264
8.65	8.65	0.323	0.323	26.3	560000	.04	1.00	1.000	0.796	3.31	0.241
4.51	4.51	0.231	0.231	32.5	136000	.05	1.00	1.000	0.755	2.74	0.276
8.66	8.66	0.279	0.279	41.2	772000	.03	1.00	1.000	0.796	3.61	0.220
8.65	8.65	0.300	0.300	48.2	1070000	.03	1.00	1.000	0.715	3.46	0.207
JSSC In-P	lane Bend	ing Tests	(WRC B	ulletin 256-	Table 4)						
D	d	т	t	Sv	Mt				B.'	CC N-392	
mm.	mm.	mm.	mm.	- , Ka/mm2	Ka/m	T/D	d/D	t/T	(S,Z,/MI)	C.	B, '/C.
164.5	42.7	4.7	3.3	48	215	.03	.26	0.702	0.835	3.64	0.229
166.5	76.3	4.5	2.9	48	640	.03	.20	0.644	0.887	2.87	0.309
316.9	60.5	4,4	4.4	45	340	.01	.19	1.000	1.343	6.40	0.210
316.9	139.8	4,4	4.4	45	1520	.01	.44	1.000	1.819	6.19	0.294
456.0	89.1	4.8	3.0	41	620	.01	.20	0.625	1.118	5.04	0.222
456.0	165.0	4.8	4.7	41	1840	.01	.36	0.979	2.055	7.23	0.284

The next column, labeled C<sub>L</sub>, lists the values of the stress indices calculated using Code Case N-392. The last column lists the ratio of  $B_L'/C_L$ . The average value is 0.227.

Table 4-2 lists similar data from for out-of-plane bending [25]. The average value of  $B_{\rm L}^{\,\prime}/\,C_{\rm L}^{\,}$  is 0.207.

D	d	Т	t	Sy	Mt				B∟'	CC N-392	
mm.	mm.	mm.	mm.	Kg/mm2	Kg/m	T/D	d/D	t/T	(S <sub>Y</sub> Z <sub>T</sub> /ML)	C <sub>N</sub>	B <sub>L</sub> '/C <sub>N</sub>
164.5	42.7	4.7	3.3	48	185	.0286	.260	0.702	0.970	4.84	0.200
166.5	76.3	4.5	2.9	48	405	.0270	.458	0.644	1.401	7.64	0.183
316.9	60.5	4.4	4.4	45	225	.0139	.191	1.000	2.029	10.36	0.196
316.9	139.8	4.4	4.4	45	675	.0139	.441	1.000	4.095	20.08	0.204
456.0	89.1	4.8	3.0	41	360	.0105	. 195	0.625	1.925	9.19	0.209
456.0	165.0	4.8	4.7	41	680	.0105	.362	0.979	5.561	22.29	0.249

 Table 4-2

 JSSC Out-of-Plane Bending Tests (WRC Bulletin 256-Table 4)

The data contains values of t/T, which are greater than the limit of applicability of N-392, which is t/T  $\leq$  1.0. The values of  $B_L'/C_L$ , where t/T > 1.0, tend to be smaller than for t/T  $\leq$  1.0.

### **Analysis of Results**

As discuss earlier, the Code Case specifies that the B indices be calculated as 0.5 times the C indices. The experimental data suggests that this is conservative. Tables 4-1 and 4-2 indicate the B indices determined experimentally from Gibstein, Toprac, and *Tubular Joints* are about 0.2 times the value of the C indices calculated using the Code Case [23,24,25]. For in-plane bending, the average value of  $B_L'/C_L$  is 0.227 for all tests. For t/T  $\leq$  1.0 the ratio is 0.248; for t/T> 1.0, it is 0.199. For out-of-plane bending, the average value is 0.207 (t/T  $\leq$  1.0).

# **5** COMPARISON OF TEST DATA TO CODE CASE RESULTS

### Purpose

The purpose of this section is to summarize the test results and compare them to those calculated by using the Code Case.

### **C** Indices

The analysis of the test data developed as a part of this study indicated that the values of the stress indices calculated by the Code Case are conservative. Two methods were used to evaluate this conservatism. The method referred to herein as the "C<sub>L</sub> Indices—Markl Approach" yields a value of C<sub>L</sub>(Code Case)/C<sub>L</sub>(test) = 1.37. For the method referred to as the "C<sub>L</sub> Indices—Class 1 Approach," C<sub>L</sub>(Code Case)/C<sub>L</sub>(test) = 1.55.

### **B** Indices

The tests performed as a part of this study were not specifically focused on the type of testing required to provide data for experimental determination of the B indices. The data was sufficient to demonstrate that the Code Case was conservative.

Additional experimental data was available. Analysis of the data indicated that the present Code Case is conservative. This was for both in-plane and out-of-plane bending of the trunnion. Based on the experimental data, the value of the B indices is about 0.21 times the value of the C index from the Code Case. The present version of the Code Case specifies that the B indices be taken as 0.5 times the C index.

If the C indices are to be reduced by a factor of 1.4, a value of the B index of 0.3 times the new C index would correspond to the test data.

Comparison of Test Data to Code Case Results

### **Loadings in Other Directions**

The tests performed for this investigation were for in-plane bending of the trunnion. As the indices for other loading conditions, given in the Code Case, were based on the same theoretical approach (finite element analysis), it is reasonable to assume that the same degree of conservatism exists for these indices. This assumption is verified by the analysis of the B indices for out-of-plane and in-plane moments, which indicated the same degree of conservatism.
# **6** FINITE ELEMENT ANALYSIS (FEA) INVESTIGATION OF FLEXIBILITY OF TRUNNIONS ON STRAIGHT PIPE

### General

This section discusses the flexibility of the trunnion-pipe configuration. This is important because trunnion-pipe configurations are often used as anchors in piping systems.

### Discussion

In piping analysis, the various components are modeled as one-dimensional beam elements. In order to accurately represent the load displacement (flexibility) action of the components, flexibility factors are used.

For bending of a straight pipe of length L, the rotation of one end,  $\phi,$  with respect to the other is

$\phi = 1/EI \int_{-L}^{L} M dx$	(Eq. 6-1)
	(19.01)

where M is the bending moment. The rotation,  $\boldsymbol{\varphi},$  is in the same direction as the applied moment.

For a torsional moment, the rotation is given by

 $\phi = 1/GJ \int_{0}^{L} M \, dx = 1.3/EI \int_{0}^{L} M_{T} \, dx$  (Eq. 6-2)

where  $M_{T}$  is the torsional moment.

The flexibility of the trunnion connection will be investigated. For configurations such as branch connections, or trunnions, the flexibility or rotation is due to local deformations in the area of intersection.

There are several possible ways to model the trunnion/pipe connection. Figure 6-1 indicates one possible model. A rigid link is used to connect point A to point B. At point B, a point spring is used to represent the local flexibility of the connection. This is similar to the standard model for branch connections, which includes the local flexibility of the connection. A rigid link is an element with infinite stiffness; the deflections (including rotations) are the same at both ends of the link.



Figure 6-1 Trunnion-Pipe Modeling

It is convenient to define the flexibility of the point spring by

$$\phi = \mathbf{k} \mathbf{M} \mathbf{d}_{o} / \mathbf{EI}_{t}$$
(Eq. 6-3)

where  $I_t$  is the section modulus of the trunnion. Then k is equivalent to the number of trunnion diameters that would be added to represent the local flexibility.

The flexibility factor for the spring will be calculated by

$$\mathbf{k} = (\phi_{\text{fea}} - \phi_{\text{b}}) / (\mathbf{Md}_{\text{o}} / \mathbf{I}_{\text{t}})$$
(Eq. 6-4)

where  $\phi_{fea}$  is the rotation from the finite element analysis (FEA) and  $\phi_{b}$  is the rotation from the beam model. This will be discussed in more detail later.

### **Finite Element Analysis**

The general layout for the FEA models is shown in Figure 6-2. Normally, straight sections of pipe (or trunnion) approximately equal to 2 diameters are used in the

models. COSMOS/M<sup>®2</sup> version 1.75 from Structural Research and Analysis Corporation was used. Shell elements were used in the models, which usually consisted of approximately 7,000 elements. A typical model is shown in Figure 6-3. The ends of the pipe and trunnion sections are connected to rigid links.





<sup>&</sup>lt;sup>2</sup> COSMOS/M is a registered trademark of Structural Research and Analysis Corporation.

Finite Element Analysis (FEA) Investigation of Flexibility of Trunnions on Straight Pipe





The material properties used in the analyses are E = 30E6, G = 12E6, and  $\mu$  = 0.28.

For evaluation of the flexibility of the configuration, several loading conditions were used. The local rotation at the juncture of the pipe and the trunnion is somewhat dependent on the boundary conditions at the ends of the pipe (points 1 and 3). It is important to recognize that in evaluating flexibility factors, there is no "conservative" value that would be applicable for all piping layouts. As an example, a high value might mean that the loads are lower in other components in a piping system than the "true" values.

Consequently the "best" value to use is the one that is most representative of the actual value. This is complicated because the flexibility is a function of the end conditions at the ends of the straight pipe. This will, of course, be a function of the layout.

As a result of this, two sets of boundary conditions were used in the evaluation. The first was where one end of the model (point 1) was fixed, and the other end (point 3) was free; the loads were applied at the end of the trunnion (point 6) and the free end of the pipe (point 3). In the second case, both ends of the pipe (points 1 and 3) were fixed, and the loads were applied at the end of the trunnion (point 6). The flexibility factors are based on the average of the results. The significance of the boundary conditions will be determined.

The 22 models are listed in Table 6-1, along with the dimensions. Other pertinent data is also included in Table 6-1. The cases listed are representative of actual usage. As an example, it is not expected that a very small trunnion would be used on a large pipe (small d/D). Table 6-1 includes the moments used in the FEA, which are based on a nominal 10 ksi stress in the pipe or trunnion.

Tabl	е	<b>6-</b> ′	1
FEA	Μ	od	els

Model	D <sub>o</sub>	т	d <sub>o</sub>	t	d./D.	t/T	D./T	d⊿/t	D	d	D/T	d/D	d/t	Ma	Mb	h	ŀ	ы	L
	(in.)	(in.)	(in.)	(in.)			, i		(in.)	(in.)				(inlbs.)	(inlbs.)	(in <sup>4</sup> )	(in <sup>4</sup> )	(in.)	(in.)
	(,	(,	(,	(,					()	()				(	(	()	()	()	()
TS1	8.625	0.250	4.50	0.237	0.522	0.948	34.5	19.0	8.375	4.263	33.5	0.51	18.0	33827	137721	57.7	7.2	19.50	15.19
TS2	12.75	0.375	4.50	0.237	0.353	0.632	34.0	19.0	12.375	4.263	33.0	0.34	18.0	33827	451036	279.3	7.2	19.50	13.13
TS3	12.75	0.375	10.75	0.365	0.843	0.973	34.0	29.5	12.375	10.385	33.0	0.84	28.5	309169	451036	279.3	160.7	19.50	13.13
TS4	12.75	0.375	10.75	0.594	0.843	1.584	34.0	18.1	12.375	10.156	33.0	0.82	17.1	481195	451036	279.3	245.2	19.50	13.13
T1	10.00	0.500	5.00	0.250	0.500	0.500	20.0	20.0	9.500	4.750	19.0	0.50	19.0	44301	354411	168.8	10.6	19.50	14.50
T2	10.00	0.500	7.50	0.375	0.750	0.750	20.0	20.0	9.500	7.125	19.0	0.75	19.0	149517	354411	168.8	53.4	19.50	14.50
Т3	10.00	0.500	8.50	0.425	0.850	0.850	20.0	20.0	9.500	8.075	19.0	0.85	19.0	217652	354411	168.8	88.1	19.50	14.50
T4	10.00	0.500	5.00	0.500	0.500	1.000	20.0	10.0	9.500	4.500	19.0	0.47	9.0	79521	354411	168.8	18.1	19.50	14.50
T5	10.00	0.500	7.50	0.750	0.750	1.500	20.0	10.0	9.500	6.750	19.0	0.71	9.0	268385	354411	168.8	91.7	19.50	14.50
T6	10.00	0.500	8.50	0.850	0.850	1.700	20.0	10.0	9.500	7.650	19.0	0.81	9.0	390689	354411	168.8	151.3	19.50	14.50
T7	10.00	0.333	5.00	0.167	0.500	0.500	30.0	30.0	9.667	4.833	29.0	0.50	29.0	30580	244637	118.4	7.4	19.50	14.50
T8	10.00	0.333	7.50	0.250	0.750	0.750	30.0	30.0	9.667	7.250	29.0	0.75	29.0	103206	244637	118.4	37.5	19.50	14.50
Т9	10.00	0.333	8.50	0.283	0.850	0.850	30.0	30.0	9.667	8.217	29.0	0.85	29.0	150238	244637	118.4	61.8	19.50	14.50
T10	10.00	0.333	5.00	0.333	0.500	1.000	30.0	15.0	9.667	4.667	29.0	0.48	14.0	57014	244637	118.4	13.4	19.50	14.50
T11	10.00	0.333	7.50	0.500	0.750	1.500	30.0	15.0	9.667	7.000	29.0	0.72	14.0	192422	244637	118.4	67.7	19.50	14.50
T12	10.00	0.333	8.50	0.567	0.850	1.700	30.0	15.0	9.667	7.933	29.0	0.82	14.0	280110	244637	118.4	111.7	19.50	14.50
T13	10.00	0.200	5.00	0.100	0.500	0.500	50.0	50.0	9.800	4.900	49.0	0.50	49.0	18857	150859	74.0	4.6	19.50	14.50
T14	10.00	0.200	7.50	0.150	0.750	0.750	50.0	50.0	9.800	7.350	49.0	0.75	49.0	63644	150859	74.0	23.4	19.50	14.50
T15	10.00	0.200	8.50	0.170	0.850	0.850	50.0	50.0	9.800	8.330	49.0	0.85	49.0	92646	150859	74.0	38.6	19.50	14.50
T16	10.00	0.200	5.00	0.200	0.500	1.000	50.0	25.0	9.800	4.800	49.0	0.49	24.0	36191	150859	74.0	8.7	19.50	14.50
T17	10.00	0.200	7.50	0.300	0.750	1.500	50.0	25.0	9.800	7.200	49.0	0.73	24.0	122145	150859	74.0	44.0	19.50	14.50
T18	10.00	0.200	8.50	0.340	0.850	1.700	50.0	25.0	9.800	8.160	49.0	0.83	24.0	177807	150859	74.0	72.7	19.50	14.50
NOTES:																			
1. See Fi	aure 6-3 for	definition	of L, and L.																
	9		5. <u>1</u> , and <u>2</u> 2																
2. The me	oments, M <sub>a</sub>	and M <sub>b</sub> , ar	re based on	a nomina	l stress of	10 ksi in th	ne pipe or t	runnion, as	ssuming a	n approxim	ate section	n modulus	ofpxme	an radius <sup>2</sup>	k thickness				
The va	lues of k are	e independ	lent of M <sub>a</sub> o	r M <sub>b</sub> .															

### **FEA Results**

Table 6-2 lists the rotations at the indicated points for the specific load cases. These rotations were taken directly from the FEA output and can be considered the rotation with respect to a fixed point. Table 6-2 and the following tables refer to  $\phi_x$ ,  $\phi_y$ , and  $\phi_z$ . These represent, respectively, the rotations around the x-, y-, and z-axes. The x-axis is along the centerline of the trunnion; the y-axis is along the centerline of the pipe, and the z-axis is perpendicular to x and y. The results are discussed in the following sections.

	In-Plane	Out-of-Plane	Torsion	In-Plane	Out-of-Plane	Torsion
	Moment on	Moment on	Moment on	Moment on	Moment on	Moment on
	Trunnion	Trunnion	Trunnion	Pipe	Pipe	Pipe
Case	1	2	3	4	5	6
Model	fz	f <sub>Y</sub>	f <sub>x</sub>	fz	f <sub>x</sub>	f <sub>Y</sub>
TS1	6.15E-03	1.68E-02	3.99E-03	1.55E-03	1.55E-03	2.03E-03
TS2	4.64E-03	9.25E-03	3.04E-03	1.05E-03	1.05E-03	1.38E-03
TS3	3.44E-03	7.53E-03	2.34E-03	1.04E-03	1.04E-03	1.36E-03
TS4	4.81E-03	1.11E-02	2.96E-03	1.04E-03	1.04E-03	1.36E-03
T1	3.46E-03	5.61E-03	3.11E-03	1.36E-03	1.37E-03	1.79E-03
T2	3.34E-03	6.11E-03	2.78E-03	1.36E-03	1.36E-03	1.78E-03
Т3	3.44E-03	5.97E-03	2.89E-03	1.36E-03	1.36E-03	1.77E-03
T4	4.66E-03	8.36E-03	3.54E-03	1.36E-03	1.37E-03	1.79E-03
T5	4.99E-03	9.87E-03	3.57E-03	1.35E-03	1.36E-03	1.78E-03
Т6	5.29E-03	9.89E-03	3.91E-03	1.34E-03	1.36E-03	1.77E-03
T7	3.84E-03	8.03E-03	3.08E-03	1.34E-02	1.35E-03	1.76E-03
Т8	3.65E-03	8.47E-03	2.77E-03	1.34E-03	1.34E-03	1.76E-03
Т9	3.70E-03	7.95E-03	2.88E-03	1.34E-03	1.34E-03	1.75E-03
T10	5.35E-03	1.26E-02	3.47E-03	1.34E-03	1.35E-03	1.76E-03
T11	5.59E-03	1.42E-02	3.57E-03	1.33E-03	1.34E-03	1.75E-03
T12	5.82E-03	1.36E-02	3.93E-03	1.33E-03	1.34E-03	1.74E-03
T13	4.48E-03	1.31E-02	3.06E-03	1.33E-03	1.33E-03	1.74E-03
T14	4.16E-03	1.25E-02	2.76E-03	1.32E-03	1.33E-03	1.74E-03
T15	4.12E-03	1.11E-02	2.87E-03	1.32E-03	1.33E-03	1.73E-03
T16	6.55E-03	2.16E-02	3.44E-03	1.32E-03	1.33E-03	1.74E-03
T17	6.52E-03	2.17E-02	3.59E-03	1.32E-03	1.33E-03	1.73E-03
T18	6.57E-03	1.95E-02	3.96E-03	1.32E-03	1.32E-03	1.73E-03
Notes:						
1. Loads	on the trunnion we	ere applied at point 6	(Figure 6-2).			
2. Loads	on the pipe were a	applied at point 3 (Fig	gure 6-2).			

## Table 6-2Summary of Rotations for Point 6—One End of Pipe Fixed

Table 6-2 indicates that the trunnion does not affect the rotations of the pipe for loads applied at the end of the pipe. This is seen by reviewing models T1–T18, where the pipe is the same size and the size of the trunnion is changed.

Figure 6-2 shows the model to be used as the basis of the evaluation of the flexibility of the trunnion. A rigid link is included in the model from the centerline of the run pipe to its surface. At that juncture, a point spring is used to represent the local flexibility of the connection. The model depicted in Figure 6-2 will be used as the basis of evaluating the FEA results for loads on the trunnion. The rotation at the end of the trunnion (point 6), with respect to the fixed point 1, is given by

$$\phi_6 = \phi_{6-5} + \phi_{5-4} + \phi_{4-2} + \phi_{2-1}$$

(Eq. 6-5)

where  $\phi_{ij}$  is the rotation of point "i" with respect to point "j," and  $\phi_6$  is the rotation of the end of the trunnion (see Figure 6-2). Note that for in-plane bending:

$$\begin{split} \varphi_{6-5} &= ML_2/(EI_t) \eqno(Eq. 6-6) \\ \varphi_{5-4} &= k \ Md_o/(EI_t) \ \text{point spring (from Equation 6-3)} \\ \varphi_{4-2} &= 0 \ (\text{because this is the rotation over a rigid link}) \\ \varphi_{2-1} &= ML_1/(EI_p) \eqno(Eq. 6-7) \\ \text{Replacing } \varphi_6 \ \text{with } \varphi_{fea}, \ \text{and rearranging yields:} \end{split}$$

$$k = 1/(Md_o/(EI_t)) \ [\phi_{fea} - \phi_{6.5} - \phi_{4.2} - \phi_{2.1}]$$
(Eq. 6-8)

For torsion of the trunnion, Equation 6-6 is replaced by:

$$\phi_{6.5} = 1.3 \text{ ML}_2 / (\text{EI}_1)$$
 (Eq. 6-9)

For out-of-plane bending of the trunnion because the segment of the beam model from point 1 to point 2 is in torsion, Equation 6-7 is replaced by

$$\phi_{2,1} = 1.3 \text{ ML}_1 / (\text{EI}_p)$$
 (Eq. 6-10)

As discussed earlier, the FEA was performed with two sets of boundary conditions, fixed at one pipe end (point 1) and fixed at both ends (points 1 and 3). When fixed at both of the pipe ends, for in-plane moments or torsion on the trunnion, Equation 6-7 is replaced by:

$$\phi_{2.1} = ML_1 / (8EI_p) \tag{Eq. 6-11}$$

For out-of-plane moments, Equation 6-7 is replaced by:

$$\phi_{2.1} = 1.3 \text{ ML}_1 / (2\text{EI}_p)$$
 (Eq. 6-12)

Table 6-3 lists the values of k for in-plane bending, out-of-plane bending, and torsion for the case with only one end fixed. For in-plane bending, the values of k range from 1.79 to 6.09. For out-of-plane bending, the values are larger with a maximum of about 27. For torsion, the values are smaller with a range of 0.43 to 1.24.

							<u>.</u> .	-	•
				In-PI	ane	Out-of-	Plane	lors	ion
				Mome	nt on	Mome	nt on	Mome	nt on
				Trun	nion	Trun	nion	Trunr	nion
Model	D/T	d/D	d/t	φz	k	ф ү	k	фx	k
TS1	33.5	0.509	18.0	6.15E-03	4.85	1.68E-02	19.79	3.99E-03	0.761
TS2	33.0	0.344	18.0	4.64E-03	3.58	9.25E-03	10.12	3.04E-03	0.429
TS3	33.0	0.839	28.5	3.44E-03	2.72	7.53E-03	8.34	2.34E-03	0.764
TS4	33.0	0.821	17.1	4.81E-03	4.03	1.11E-02	12.52	2.96E-03	1.032
T1	19.0	0.500	19.0	3.46E-03	1.79	5.61E-03	4.80	3.11E-03	0.435
T2	19.0	0.750	19.0	3.34E-03	2.02	6.11E-03	5.73	2.78E-03	0.636
Т3	19.0	0.850	19.0	3.44E-03	2.01	5.97E-03	5.26	2.89E-03	0.712
Τ4	19.0	0.474	9.0	4.66E-03	3.04	8.36E-03	7.98	3.54E-03	0.650
Т5	19.0	0.711	9.0	4.99E-03	3.47	9.87E-03	9.72	3.57E-03	0.957
Т6	19.0	0.805	9.0	5.29E-03	3.46	9.89E-03	9.13	3.91E-03	1.066
T7	29.0	0.500	29.0	3.84E-03	2.43	8.03E-03	8.45	3.08E-03	0.453
Т8	29.0	0.750	29.0	3.65E-03	2.55	8.47E-03	9.29	2.77E-03	0.678
Т9	29.0	0.850	29.0	3.70E-03	2.47	7.95E-03	8.28	2.88E-03	0.760
T10	29.0	0.483	14.0	5.35E-03	4.19	1.26E-02	14.30	3.47E-03	0.678
T11	29.0	0.724	14.0	5.59E-03	4.44	1.42E-02	16.10	3.57E-03	1.029
T12	29.0	0.821	14.0	5.82E-03	4.32	1.36E-02	14.67	3.93E-03	1.154
T13	49.0	0.500	49.0	4.48E-03	3.44	1.31E-02	16.00	3.06E-03	0.482
T14	49.0	0.750	49.0	4.16E-03	3.36	1.25E-02	15.39	2.76E-03	0.720
T15	49.0	0.850	49.0	4.12E-03	3.15	1.11E-02	13.00	2.87E-03	0.807
T16	49.0	0.490	24.0	6.55E-03	6.09	2.16E-02	27.68	3.44E-03	0.739
T17	49.0	0.735	24.0	6.52E-03	5.92	2.17E-02	27.30	3.59E-03	1.109
T18	49.0	0.833	24.0	6.57E-03	5.52	1.95E-02	23.48	3.96E-03	1.236

## Table 6-3Bending of the Trunnion—One Pipe End Fixed

Table 6-4 lists the values of k for the condition with both ends fixed. The values are very close to the condition with only one end of the pipe fixed. Tables 6-5 through 6-7 include a comparison of the results for the two sets of boundary conditions. The variations are within the analysis methodology tolerance.

				In-Pla	ine	Out-of-	Plane	Tors	sion
				Momer	nt on	Mome	nt on	Mome	nt on
				Trunn	ion	Trun	nion	Trun	nion
Model	D/T	d/D	d/t	φz	k	фу	k	фx	k
TS1	33.5	0.509	18.0	5.53E-03	4.43	1.50E-02	17.67	3.62E-03	0.71
TS2	33.0	0.344	18.0	4.56E-03	3.57	9.20E-03	10.12	2.98E-03	0.43
TS3	33.0	0.839	28.5	2.78E-03	2.68	7.05E-03	8.33	1.72E-03	0.77
TS4	33.0	0.821	17.1	3.80E-03	3.98	1.04E-02	12.50	1.99E-03	1.05
T1	19.0	0.500	19.0	3.30E-03	1.78	5.50E-03	4.79	2.97E-03	0.44
T2	19.0	0.750	19.0	2.82E-03	1.99	5.73E-03	5.72	2.28E-03	0.65
Т3	19.0	0.850	19.0	2.69E-03	1.98	5.42E-03	5.26	2.17E-03	0.73
Τ4	19.0	0.474	9.0	4.38E-03	3.03	8.16E-03	7.98	3.28E-03	0.65
T5	19.0	0.711	9.0	4.06E-03	3.44	9.19E-03	9.70	2.68E-03	0.98
Т6	19.0	0.805	9.0	3.95E-03	3.44	8.90E-03	9.13	2.62E-03	1.10
T7	29.0	0.500	29.0	3.68E-03	2.42	7.92E-03	8.44	2.93E-03	0.46
Т8	29.0	0.750	29.0	3.14E-03	2.52	8.10E-03	9.29	2.28E-03	0.69
Т9	29.0	0.850	29.0	2.96E-03	2.45	7.41E-03	8.27	2.17E-03	0.78
T10	29.0	0.483	14.0	5.07E-03	4.18	1.24E-02	14.29	3.20E-03	0.68
T11	29.0	0.724	14.0	4.65E-03	4.42	1.35E-02	16.08	2.66E-03	1.05
T12	29.0	0.821	14.0	4.45E-03	4.29	1.35E-02	15.94	2.61E-03	1.19
T13	49.0	0.500	49.0	4.33E-03	3.43	1.30E-02	16.00	2.92E-03	0.49
T14	49.0	0.750	49.0	3.66E-03	3.34	1.21E-02	15.39	2.28E-03	0.73
T15	49.0	0.850	49.0	3.38E-03 3.12		1.05E-02	12.99	2.17E-03	0.82
T16	49.0	0.490	24.0	6.26E-03 6.08		2.14E-02	2.14E-02 27.67		0.75
T17	49.0	0.735	24.0	5.56E-03 5.89		2.09E-02	27.27	2.66E-03	1.13
T18	49.0	0.833	24.0	5.22E-03	5.54	1.86E-02	23.67	2.62E-03	1.28

# Table 6-4Bending of the Trunnion—Both Ends Fixed

				1 End	2 Ends				
				Fixed F k /t Note (1) No			Average		
				k	k		k	R EQ	
Model	D/T	d/D	d/t	Note (1)	Note (1)	% Diff	Note (1)	Note (2)	% Diff
TS1	33.5	0.509	18.0	4.85	4.43	-8.5	4.64	4.16	-10.3
TS2	33.0	0.344	18.0	3.58	3.57	-0.2	3.58	3.99	11.5
TS3	33.0	0.839	28.5	2.72	2.68	-1.6	2.70	2.96	9.8
TS4	33.0	0.821	17.1	4.03	3.98	-1.1	4.01	4.36	8.8
T1	19.0	0.500	19.0	1.79	1.78	-0.5	1.79	1.86	3.8
T2	19.0	0.750	19.0	2.02	1.99	-1.1	2.01	1.90	-5.3
Т3	19.0	0.850	19.0	2.01	1.98	-1.3	2.00	1.91	-4.1
T4	19.0	0.474	9.0	3.04	3.03	-0.4	3.04	3.26	7.4
Т5	19.0	0.711	9.0	3.47	3.44	-0.7	3.46	3.34	-3.3
Т6	19.0	0.805	9.0	3.46	3.44	-0.6	3.45	3.37	-2.5
T7	29.0	0.500	29.0	2.43	2.42	-0.4	2.42	2.38	-1.7
Т8	29.0	0.750	29.0	2.55	2.52	-0.9	2.54	2.44	-3.8
Т9	29.0	0.850	29.0	2.47	2.45	-1.1	2.46	2.46	-0.2
T10	29.0	0.483	14.0	4.19	4.18	-0.3	4.18	4.13	-1.2
T11	29.0	0.724	14.0	4.44	4.42	-0.6	4.43	4.23	-4.5
T12	29.0	0.821	14.0	4.32	4.29	-0.6	4.30	4.26	-1.0
T13	49.0	0.500	49.0	3.44	3.43	-0.3	3.44	3.25	-5.6
T14	49.0	0.750	49.0	3.36	3.34	-0.7	3.35	3.32	-0.9
T15	49.0	0.850	49.0	3.15	3.12	-0.9	3.14	3.35	6.7
T16	49.0	0.490	24.0	6.09	6.08	-0.2	6.08	5.58	-8.4
T17	49.0	0.735	24.0	5.92	5.89	-0.4	5.91	5.71	-3.4
T18	49.0	0.833	24.0	5.52	5.54	0.4	5.53	5.75	4.0
					Average =	-1.0		Average =	-0.2
			N	/laximum =	0.4	r	/laximum =	11.5	
				Minimum =	-8.5		Minimum =	-10.3	
Notes:					STD=	1.73		STD=	6.00
1. kb	. k based on finite element analysis (Fl								
2. kb	ased on regi	ression Equ	ation 6-13						

# Table 6-5In-Plane Bending of the Trunnion—Average of Boundary Conditions

				1 End	2 Ends						
				Fixed	Fixed		Average	w/o factor		w/ factor	
				k	k		k	R EQ		R EQ	
Model	D/T	d/D	d/t	Note (1)	Note (1)	% Diff	Note (1)	Note (2)	% Diff	Note(3)	% Diff
TS1	33.5	0.509	18.0	19.79	17.67	-10.7	18.73	14.99	-19.9	15.4	-17.5
TS2	33.0	0.344	18.0	10.12	10.12	0.0	10.12	14.08	39.1	12.3	21.8
TS3	33.0	0.839	28.5	8.34	8.33	-0.1	8.34	10.82	29.8	10.4	24.6
TS4	33.0	0.821	17.1	12.52	12.50	-0.1	12.51	15.81	26.4	15.5	23.6
T1	19.0	0.500	19.0	4.80	4.79	-0.1	4.80	5.03	4.9	5.1	7.0
T2	19.0	0.750	19.0	5.73	5.72	-0.1	5.73	5.22	-8.9	5.3	-6.8
Т3	19.0	0.850	19.0	5.26	5.26	-0.1	5.26	5.28	0.4	5.0	-5.2
T4	19.0	0.474	9.0	7.98	7.98	-0.1	7.98	8.77	9.9	8.8	10.2
T5	19.0	0.711	9.0	9.72	9.70	-0.1	9.71	9.10	-6.3	9.5	-2.4
Т6	19.0	0.805	9.0	9.13	9.13	-0.1	9.13	9.20	0.8	9.1	-0.7
T7	29.0	0.500	29.0	8.45	8.44	0.0	8.44	8.01	-5.1	8.2	-2.9
T8	29.0	0.750	29.0	9.29	9.29	-0.1	9.29	8.31 -10.5		8.5	-8.1
Т9	29.0	0.850	29.0	8.28	8.27	-0.1	8.27	8.40 1.6		8.0	-3.6
T10	29.0	0.483	14.0	14.30	14.29	-0.1	14.29	13.79	-3.5	14.0	-2.3
T11	29.0	0.724	14.0	16.10	16.08	-0.1	16.09	14.30	-11.1	14.9	-7.5
T12	29.0	0.821	14.0	14.67	15.94	8.6	15.31	14.47	-5.5	14.1	-7.7
T13	49.0	0.500	49.0	16.00	16.00	0.0	16.00	14.27	-10.8	14.7	-8.3
T14	49.0	0.750	49.0	15.39	15.39	-0.1	15.39	14.80	-3.8	15.3	-0.7
T15	49.0	0.850	49.0	13.00	12.99	-0.1	12.99	14.97	15.2	14.3	9.8
T16	49.0	0.490	24.0	27.68	27.67	0.0	27.67	24.32	-12.1	24.9	-10.1
T17	49.0	0.735	24.0	27.30	27.27	-0.1	27.28	25.23	-7.5	26.3	-3.7
T18	49.0	0.833	24.0	23.48	23.67	0.8	23.58	25.51	8.2	24.8	5.0
					Average =	-0.1		Average =	1.4	Average =	0.6
				1	Maximum =	8.6		Maximum =	39.1	laximum =	24.6
					Minimum =	-10.7		Minimum =	-19.9	/linimum =	-17.5
Notes:					STD=	3.01		STD=	14.87	STD=	11.30
1. k base	ed on FE	A									
2. k base	ed on reg	ression Ec	quation 6-	14							
3. k based on regression Equation		uation 6-	15								

Table 6-6Out-of-Plane Bending of the Trunnion—Average of Boundary Conditions

				1 End	2 Ends				
				Fixed	Fixed		Average		
				k	k		k	R EQ	
Model	D/T	d/D	d/t	Note (1)	Note (1)	% Diff	Note (1)	Note (2)	% Diff
TS1	33.5	0.509	18.0	0.76	0.71	-6.7	0.74	0.68	-7.3
TS2	33.0	0.344	18.0	0.43	0.43	1.3	0.43	0.46	5.7
TS3	33.0	0.839	28.5	0.76	0.77	0.9	0.77	0.84	9.4
TS4	33.0	0.821	17.1	1.03	1.05	1.6	1.04	1.12	7.6
T1	19.0	0.500	19.0	0.43	0.44	0.7	0.44	0.42	-2.9
T2	19.0	0.750	19.0	0.64	0.65	1.5	0.64	0.63	-1.0
Т3	19.0	0.850	19.0	0.71	0.73	2.3	0.72	0.72	-0.1
T4	19.0	0.474	9.0	0.65	0.65	0.8	0.65	0.63	-3.0
T5	19.0	0.711	9.0	0.96	0.98	2.0	0.97	0.95	-1.9
T6	19.0	0.805	9.0	1.07	1.10	3.2	1.08	1.07	-0.8
T7	29.0	0.500	29.0	0.45	0.46	0.6	0.45	0.45	-1.2
T8	29.0	0.750	29.0	0.68	0.69	1.5	0.68	0.67	-1.4
Т9	29.0	0.850	29.0	0.76	0.78	2.1	0.77	0.76	-0.6
T10	29.0	0.483	14.0	0.68	0.68	0.8	0.68	0.68	-0.6
T11	29.0	0.724	14.0	1.03	1.05	2.0	1.04	1.01	-2.5
T12	29.0	0.821	14.0	1.15	1.19	2.8	1.17	1.15	-1.8
T13	49.0	0.500	49.0	0.48	0.49	0.9	0.48	0.48	-0.1
T14	49.0	0.750	49.0	0.72	0.73	1.3	0.72	0.72	0.0
T15	49.0	0.850	49.0	0.81	0.82	1.7	0.81	0.82	0.9
T16	49.0	0.490	24.0	0.74	0.75	0.8	0.74	0.73	-1.4
T17	49.0	0.735	24.0	1.11	1.13	2.1	1.12	1.10	-2.1
T18	49.0	0.833	24.0	1.24	1.28	3.7	1.26	1.24	-1.2
					Average =	1.3		Average =	-0.3
					Maximum =	3.7		Maximum =	9.4
				Minimum =	-6.7		Minimum =	-7.3	
Notes:					STD=	1.96		STD=	3.63
1. k bas	ed on FEA								
2. k bas	ed on regre	ession Equa	ation 6-16						

Table 6-7Torsion of the Trunnion—Average of Boundary Conditions

It is assumed that the average of the two conditions is representative of actual applications. Tables 6-5 through 6-7 list the average values of k and also provide a comparison to equations developed from regression analysis for the specific loading conditions on the trunnion:

In-plane bending:

$$k = 0.34 (D/T)^{1.351} (d/D)^{0.058} (d/t)^{-0.76}$$
 (Eq. 6-13)

The average difference between Equation 6-13 and the FEA results is 0.2% with the maximum abut 11.5%. The standard deviation of the percentage difference was 6.0%.

The parameter  $r^2$  is a standard statistical measure of "goodness of fit." The closer to 1.0, the more accurate the curve fit is.

Out-of-plane bending:

 $k = 0.21 (D/T)^{1.85} (d/D)^{0.09} (d/t)^{-0.75}$  (r<sup>2</sup> = 0.92) (Eq. 6-14)

The average difference between Equation 6-14 and the FEA results is 1.4%, with the maximum about 39.1%. The standard deviation of the percentage difference was 14.9%.

In order to reduce the difference between the correlation equation and the FEA results, a factor was added to account for the effects of d/D for out-of-plane bending. This resulted in the following expression for out-of-plane bending:

 $k=0.321 (1.47 (d/D)-(d/D)^{2.45}) (D/T)^{1.86} (d/D)^{-0.14} (d/t)^{-0.75} (r^{2}=0.95) (Eq. 6-15)$ 

The average difference between Equation 6-15 and the FEA results is 0.6%, with the maximum about 24.6%. The standard deviation of the percentage difference was 11.3%.

This represents a slight improvement over Equation 6-14.

Torsion:

$$k = 0.56 (D/T)^{0.75} (d/D)^{0.998} (d/t)^{-0.61}$$
 (r<sup>2</sup> = 0.99) (Eq. 6-16)

The average difference between Equation 6-16 and the FEA results is 0.2%, with the maximum abut 9.4%. The standard deviation of the percentage difference was 3.6%.

In the FEA, the range of the parameters used to develop Equations 6-13 through 6-16 were: D/T from 19 to 49, d/D from 0.34 to 0.85, and d/t from 9 to 49. It is not possible to analyze all possible combinations of these parameters. In order to determine the applicability of the various equations over the various ranges of parameters, trial calculations were performed to determine the reasonableness of the results. The calculations yielded reasonable results; hence, these parameter ranges will be used as limits in the applicability of the equations.

## **Comparison to Test Data**

While the tests discussed in Chapter 3 were not specifically for determining flexibility factors, they can be used to evaluate one of the equations derived above. Loads and deflections at the load point for in-plane bending were recorded and are included in Appendix C. The average deflection of the four tests at a load of 1,000 lbs. was 0.54 inches. Using an average trunnion length of 46 inches and other dimensions from Figure 3-1 and Equation 6-13 (for in-plane bending), the calculated deflection was 0.46 inches. Considering that the calculation does not include the flexibility of the

testing frame or bolted connections—and also that Equation 6-13 is based on the average of the two cases with different conditions—this is considered as verification of the methodology.

# 7 CONCLUSIONS

The conclusions arrived at from the analyses and tests discussed in this report are enumerated below:

- 1. The basic approach used by Code Cases N-391 and N-392 can be used in the design and qualification of trunnions on pipe.
- 2. A more accurate evaluation of trunnions or hollow circular cross section welded on pipe can be made if the tables in Code Cases N-392 and N-392 are modified such that the present values of  $A_0$  are reduced by a factor of 1.4.
- 3. The values of the B indices ( $B_w$ ,  $B_L$ ,  $B_N$ , and  $B_T$ ) are defined as 0.3 times the corresponding C index ( $C_w$ ,  $C_L$ ,  $C_N$ , and  $C_T$ ) but not less than 1.0.
- 4. For Code Case N-391, the lower limit on  $\gamma = R_{o}/T$  can be taken as 4.0 instead of 8.33.
- 5. The flexibility model should be based on Figure 6-1, in which it is assumed that there is a rigid link from the centerline of the pipe to its outer surface where the trunnion is connected. At this location, it is assumed that a point spring exists. The flexibility factors of the point spring are given by:

In-plane bending:

 $k = 0.34 (D/T)^{1.351} (d/D)^{0.058} (d/t)^{-0.76}$ (Eq. 6-13)

Out-of-plane bending:

$$k = 0.321 (1.47 (d/D) - (d/D)^{2.45}) (D/T)^{1.86} (d/D)^{-0.14} (d/t)^{-0.75}$$
(Eq. 6-15)

Torsion:

$$k = 0.56 (D/T)^{0.75} (d/D)^{0.998} (d/t)^{-0.61}$$
(Eq. 6-16)

For through-run moments, the flexibility is the same as for the run pipe without a trunnion.

Conclusions

The applicability of these equations is limited to the following range of parameters:

D/T: from 19.0 to 49.0 d/D: from 0.34 to 0.85 d/t: from 9.0 to 49.0

The approach suggested should allow for a more accurate evaluation of trunnions on straight pipe.

# **8** REFERENCES

- 1. ASME Boiler and Pressure Vessel Code, Section III, Nuclear Power Plant Components, American Society of Mechanical Engineers, New York.
- 2. American National Standards Institute (ANSI), Code for Pressure Piping, B31.1, Power Piping, American Society of Mechanical Engineers, New York.
- 3. Case N-391-2, Procedure for Evaluation of the design of Hollow Circular Cross Section Welded Attachments on Class 1 Piping, Section III, Division 1, American Society of Mechanical Engineers, New York.
- 4. Case N-392-3, Procedure for Evaluation of the design of Hollow Circular Cross Section Welded Attachments on Classes 2 and 3 Piping, Section III, Division 1, American Society of Mechanical Engineers, New York.
- 5. Rodabaugh, E.C., "Background of ASME Code Cases N-391 and N-392 Trunnions of Straight Pipe," September 1990, Attachment to ASME Code Committee, Working Group on Piping Design (WGPD) meeting minutes, April 1991.
- 6. Potvin, A.B., Kuang, J.G., Leick, R.D. and Kablich, J. L., "Stress Concentration in Tubular Joints," *Society of Petroleum Engineers Journal*, pp. 287-299, August 1977.
- 7. Dodge, W.G., "Secondary Stress Indices for Integral Structural Attachments to Straight Pipe," and Rodabaugh, E.C., Dodge, W.G., and Moore, S.E., "Stress Indices at Lug Supports on Piping Systems," *Welding Research Council Bulletin* 198, September 1974.
- 8. Mershon, J.L., Mokhtarian, K., Ranjan, G.V., and Rodabaugh, E.C., "Local Stresses in Cylindrical Shells due to External Loadings on Nozzles-Supplement to WRC Bulletin No. 107," *Welding Research Council Bulletin* 297, August 1984.
- 9. Wordsworth, A.C., and Smedley, G.P., *Stress Concentrations of Unstiffened Tubular Joints,* European Offshore Steels Research Seminar, Cambridge, 1978.
- 10. Rodabaugh, E.C. and Moore, S.E., "Stress Indices and Flexibility Factors for Nozzles in Pressure Vessels and Piping," NUREG/CR-0778, June 1979.
- 11. Rodabaugh, E.C., "Review of Data Relevant to the Design of Tubular Joints in Fixed Offshore Platforms," *Welding Research Council Bulletin* Number 256, January 1980.

- Kurobane, Y., Makino, Y. and Mitsui, Y. "Re-analysis of Ultimate Strength Data for Truss Connections in Circular Hollow Sections," International Institute of Welding, IIW Doc. XV-461-80.
- 13. Markl, A.R.C., "Fatigue Tests of Piping Components," ASME paper no. 51-PET-21, May 21, 1951.
- 14. Slagis, G., "Commentary on the 1987 Section III Attachment Rules," PVP Vol. 169, ASME, 1989.
- 15. Hankinson, R.F., Van Duyne, D.A., Stout, D. H., and Tang, Y.K., "Design Guidance for Integral Welded Attachments," PVP-Vol. 237-2, ASME, 1992.
- 16. Hankinson, R.F. and Weiler, R.A., "A Review, Discussion, and Comparison of Circular Trunnion Attachments to Piping," PVP-Vol. 218, ASME, 1991.
- 17. Melworm, R.F. and Berman, I., *Welded Attachments to Tubes—Experimentation, Analysis and Design*, Welding Research Supplement, October 1966.
- 18. Sadd, M.H. and Avent, R.R., "Stress Analysis and Stress Index Development for a Trunnion Pipe Support," *Journal of Pressure Vessel Technology*, May 1982.
- 19. Gray, M.A. and Roarty, "Finite Element Analysis of Piping Trunnions for Fatigue Loadings," PVP Vol. 120, ASME, 1987.
- 20. Basavaraju, C., Kalavar, S.R., and Chern, C.Y., "Local Stresses in Piping at Integral Attachments by Finite Element Method," PVP-Vol. 235, ASME, 1992.
- 21. Rodabaugh, E. C., "Developing Stress Intensification Factors: (1) Standardized Method for Developing Stress Intensification Factors for Piping Components," *Welding Research Council Bulletin* N-392, June 1994.
- 22. Rawls, G.B., Wais, E.A., and Rodabaugh, E.C., "Evaluation of the Capacity of Welded Attachments to Elbows as compared to the Methodology of ASME Code Case N-318," PVP-Vol. 237-2, ASME, 1992.
- 23. Gibstein, M. B., *The Static Strength of T-Joints Subjected to In-Plane Bending*, Det Norske Veritas Report No. 76-137, 7.4.76.
- 24. Toprac, A.A., "An Investigation of Welded Steel Pipe Connections," *Welding Research Council Bulletin* 71, August 1961.
- 25. *Study on Tubular Joints Used for Marine Structures*, (in Japanese), The Society of Steel Construction of Japan, (JSSC), March 1972.
- 26. Criteria of the ASME Boiler and Pressure Vessel Code for Design by Analysis in Sections III and VIII, Division 2, American Society of Mechanical Engineers, New York, 1969.

# A CASE N-391-2, PROCEDURE FOR EVALUATION OF THE DESIGN OF HOLLOW CIRCULAR CROSS SECTION WELDED ATTACHMENTS ON CLASS 1 PIPING, SECTION III, DIVISION 1, AMERICAN SOCIETY OF MECHANICAL ENGINEERS, NEW YORK.

CASE

### N-391-2

CASES OF ASME BOILER AND PRESSURE VESSEL CODE

Approval Date: August 24, 1995 See Numerical Index for expiration and any reaffirmation dates.

Case N-391-2 Procedure for Evalu

Procedure for Evaluation of the Design of Hollow Circular Cross Section Welded Attachments on Class 1 Piping Section III, Division 1

Inquiry: What procedure may be used to evaluate the design of hollow circular cross section welded attachments on Class 1 pipe under Section III, Division 1?

*Reply:* It is the opinion of the Committee that the procedures listed below may be used to evaluate the design of hollow circular cross section welded attachments on Class 1 pipe under Section III, Division 1.

#### **1.0 LIMITATIONS OF APPLICABILITY**

**1.1** The attachment shall be welded to the pipe by a full penetration weld (see Fig. 1).

**1.2** The attachment material and pipe material shall have essentially the same moduli of elasticity and coefficients of thermal expansion.

**1.3** The constants, defined in Section 2.0, fall within the following ranges:

(a)  $4.0 \le \gamma \le 50.0$ 

(b)  $0.2 \le \tau \le 1.0$ 

(c)  $0.3 \le \beta \le 1.0$ , and

(d) the axis of the attachment is perpendicular to the axis of the run pipe.

1.4 The attachment shall be made on straight pipe with the nearest edge of the attachment weld located at a minimum distance of  $\sqrt{RT}$  from any other weld or discontinuity (see Section 2.0 for definitions of <u>R</u> and T). For multiple attachments located at a distance less than  $\sqrt{RT}$  to each other, the stress effects for each individual attachment shall be superimposed.

#### 2.0 NOMENCLATURE AND DEFINITIONS (See Fig. 2)

 $R_o =$  run pipe outside radius, in.  $r_o =$  attachment outside radius, in.  $r_i =$  attachment inside radius, in. T = run pipe wall thickness, in. t = attachment wall thickness, in.  $D_o = \text{outside diameter of the run pipe, in.}$   $d_o = \text{outside diameter of the attachment, in.}$   $\frac{R}{T} = \text{mean radius of run pipe, in.}$   $A_T = \pi (r_o^2 - r_i^2)$   $Z_T = I_T/r_o$   $I_T = \frac{\pi}{4} (r_o^4 - r_i^4)$   $\underline{A} = \frac{\pi}{2} (r_o^2 - r_i^2)$   $\underline{J} = \text{lesser of } \pi r_o^2 T \text{ or } Z_T$   $\gamma = R_o/T$   $\tau = i/T$   $\beta = d_o/D_o$  $C = A_o (2\gamma)^n \beta^n 2\tau^n 3 \text{ but not less than } 1.0$ 

The equation for C shall be used to determine  $C_W$ ,  $C_L$ , and  $C_N$ , based on the following table. Select the maximum value of the pipe and the attachment equations.

Index	Part	β range	Ao	$n_1$	n <sub>2</sub>	nz
Cw	Pipe	0.3-1.0	1.40	0.81	(a)	1.33
	attachment	0.3-1.0	4.00	0.55	(b)	1.00
C,	Fipe	0.3-1.0	0.46	0.60	-0.04	0.86
L	attachment	0.3-1.0	1.10	0.23	-0.38	0.38
C∧	Pipe	0.3-0.55	0.51	1.01	0.79	0.89
	attachment	0.3-0.55	0.84	0.85	0.80	0.54
	Pipe	>0.55-1.0	0.23	1.01	-0.62	0.89
	attachment	>0.55-1.0	0.44	0.85	-0.28	0.54

NOTES:

(a) Replace  $\beta^{n_2}$  with  $e^{-l.2\beta^3}$ .

(b) Replace  $\beta^n$  with  $e^{-t.35\beta^3}$ .

 $C_T = 1.0$  for  $\beta \le 0.55$   $C_T = C_N$  for  $\beta = 1.0$ , but not less than 1.0;  $C_T$  should be linearly interpolated for  $0.55 < \beta < 1.0$ , but not less than 1.0

 $B_W = 0.5$  ( $C_W$ ), but not less than 1.0

 $B_L = 0.5$  ( $C_L$ ), but not less than 1.0

 $B_N = 0.5$  ( $C_N$ ), but not less than 1.0

 $B_T = 0.5$  ( $C_T$ ), but not less than 1.0  $K_T = 1.8$  for full penetration welds

 $S_m$  = allowable design stress intensity, psi (lesser of attachment or pipe material)

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CASE (continued)

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CASES OF ASME BOILER AND PRESSURE VESSEL CODE

- $S_{y}$  = yield stress at temperature, psi (lesser of attachment or pipe material)
- $M_L$  = bending moment applied to the attachment as shown in Fig. 2, in.-lb
- $M_N$  = bending moment applied to the attachment as shown in Fig. 2, in.-lb
- $M_T$  = bending moment applied to the attachment as shown in Fig. 2, in.-lb
- $Q_1$  = shear load applied to the attachment as shown in Fig. 2, lb
- $Q_2$  = shear load applied to the attachment as shown in Fig. 2, lb
- W = thrust load applied to the attachment as shown in Fig. 2, lb

 $M_L$ ,  $M_N$ ,  $M_T$ ,  $Q_1$ ,  $Q_2$ , and W are determined at the surface of the pipe.

 $M_L^{**}, M_N^{**}, M_T^{**}, Q_1^{**}, Q_2^{**}$ , and  $W^{**}$  are absolute values of maximum loads occurring simultaneously under all service loading conditions.

- $T_r$  = average temperature of that portion of the attachment within a distance of 2*t* from the surface of the pipe, °F
- $T_{\mu\nu}$  = average temperature of the portion of the pipe under the attachment and within a distance of  $\sqrt{RT}$  from the edge of the attachment, °F
- $E\alpha$  = modulus of elasticity, *E*, times the mean coefficient of thermal expansion,  $\alpha$ , both at room temperature, psi/°F

#### 3.0 EVALUATION PROCEDURE

The loads on the attachment cause stresses in the pipe wall. Equations are provided in 3.1(a) to determine these stresses. The attachment stresses are then added to the piping system stresses at the attachment. The piping system stresses are determined by NB-3652 Eq. (9), NB-3653.1 Eq. (11) NB-3653.2 Eq. (11), and NB-3653.6 Eqs. (12), (13), and (14). The Code equations including the attachment stress terms are given in 3.1(b). The attachment stresses  $S_{MT}$ ,  $S_{MP}$ , and  $S_{PT}$  are to be calculated for the loading conditions corresponding to NB-3652 Eq. (9), NB-3653.1 Eq. (10), NB-3653.2 Eq. (11), and NB-3653.6 Eqs. (12), (13), and (14). For example, in calculating  $S_{MT}$  for use in NB-3652 Eq. (9) for design conditions, W,  $M_L$ ,  $M_N$ ,  $Q_1$ ,  $Q_2$ , and  $M_T$  are the loads on the attachment due to design mechanical loads.

There are additional equations given in 3.1(c) that also must be checked for attachment stresses. These are based on the absolute values of maximum loads occurring simultaneously under all service loading conditions.

#### 3.1 Analysis of Attachments

(a) Calculate the stresses  $S_{MT}$ ,  $S_{NT}$ ,  $S_{PT}$ , and  $S_{NT}^{**}$ :

$$S_{MT} = \frac{B_{W} W}{A_{T}} + \frac{B_{N} M_{N}}{Z_{T}} + \frac{B_{L} M_{L}}{Z_{T}} + \frac{Q_{1}}{\overline{A}} + \frac{Q_{2}}{\overline{A}} + \frac{B_{T} M_{T}}{\overline{T}}$$
(1)

$$S_{NT} = \frac{C_W W}{A_r} + \frac{C_N M_N}{Z_r} + \frac{C_L M_L}{Z_r} + \frac{Q_1}{\overline{A}} + \frac{Q_2}{\overline{A}} + \frac{Q_2}{\overline{A}} + \frac{C_T M_T}{\overline{J}} + 1.7 E \alpha (T_r - T_W)$$
(2)

$$S_{rr} = K_r(S_{NT}) \tag{3}$$

$$S_{NT}^{**} = \frac{C_{1P}W^{**}}{A_{T}} + \frac{C_{N}M_{N}^{**}}{Z_{T}} + \frac{C_{L}M_{L}^{**}}{Z_{T}} + \frac{Q_{1}^{**}}{\overline{A}} + \frac{Q_{2}^{**}}{\overline{A}} + \frac{C_{T}M_{T}^{**}}{\overline{J}}$$
(4)

(b) The following modified Code equations shall be satisfied.

(1) NB-3652 Eq. (9) becomes:

$$\frac{D_{e}}{2t} + B_{2}\frac{D_{e}}{2l}M_{t} + S_{Atr} \leq 1.5S_{m} \text{ for Design and Service} \\ \text{Level A loadings} \\ \leq 1.8S_{m} \text{ but not greater than} \\ 1.5S_{s} \text{ for Level B} \\ \text{loadings} \\ \leq 2.25S_{m} \text{ but not greater than} \\ 1.8S_{s} \text{ for Level C} \\ \text{loadings} \\ \leq 3.0S_{m} \text{ but not greater than} \\ 2.0S_{s} \text{ for Level D} \\ \text{loadings} \end{cases}$$

where  $B_1 = 0.5$  and  $B_2 = 1.0$  for straight pipe. (2) NB-3653.1 Eq. (10) becomes:

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 $B_1 \stackrel{P}{=}$ 

<sup>&#</sup>x27;For thermal transients with fluid temperature changes greater than 100°F (37.8°C) and rate of change greater than 10°F/min. IT<sub>T</sub> - T<sub>w</sub>I may be conservatively taken as one-half of the difference between the initial metal temperature and the transient fluid temperature during a temperature transient.

CASE

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#### CASES OF ASME BOILER AND PRESSURE VESSEL CODE

$$S_n = C_1 \frac{P_o D_o}{2t} + C_2 \frac{D_o}{2I} M_i + S_{NT} \le 3S_m$$
 (NB-10)

where  $C_1 = C_2 = 1.0$  for straight pipe.

If  $S_n$ , as calculated by Eq. (NB-10), exceeds  $3S_m$ , NB-3653.6 Eqs. (12) and (13) and the thermal stress ratchet check of NB-3653.7 must be satisfied;  $S_{NT}$  need not be included in these checks. However, the value of  $K_e$  shall be determined from  $S_n$ , including  $S_{NT}$ .

(3) NB-3653.2 Eq. (11) becomes:

$$S_{p} = K_{1}C_{1}\frac{P_{o}D_{o}}{2t} + K_{2}C_{2}\frac{D_{o}}{2I}M_{i} + \frac{1}{2(1-\nu)}K_{3}E\alpha|\Delta T_{1}| + \frac{1}{1-\nu}E\alpha|\Delta T_{2}| + S_{rr} \quad (NB-11)$$

where  $K_1 = K_2 = K_3 = 1.0$  for straight pipe. (4) NB-3653.6 Eq. (14) becomes:

$$S_{ALT} = K_{\epsilon} \frac{S_{p}}{2}$$
 (NB-14)

where  $S_n$  and  $S_p$  are as calculated by Eqs. (NB-10) and (NB-11) of this Case.

All terms except attachment stresses, or where otherwise noted, are defined in NB-3652 and NB-3653. (c) In addition to the Code equations, the follow-

ing equations shall also be satisfied:

$$S_{NT}^{**} \leq 2S_{y} \tag{5}$$

$$\frac{Q_1^{**}}{\overline{A}} + \frac{Q_2^{**}}{\overline{A}} + \frac{M_T^{**}}{\overline{J}} \leq S_y \tag{6}$$

#### 3.2 Analysis

Analysis complying with this Case shall be included in the Design Report for the piping system.

#### 4.0 IDENTIFICATION

This Case number shall be shown on the Data Report Form.

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CASE (continued)

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CASES OF ASME BOILER AND PRESSURE VESSEL CODE





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# B

CASE N-392-3, PROCEDURE FOR EVALUATION OF THE DESIGN OF HOLLOW CIRCULAR CROSS SECTION WELDED ATTACHMENTS ON CLASSES 2 AND 3 PIPING, SECTION III, DIVISION 1, AMERICAN SOCIETY OF MECHANICAL ENGINEERS, NEW YORK

CASE

N-392-3

CASES OF ASME BOILER AND PRESSURE VESSEL CODE

#### Approval Date: December 12, 1994

See Numerical Index for expiration and any reaffirmation dates.

Case N-392-3

Procedure for Evaluation of the Design of Hollow Circular Cross Section Welded Attachments on Classes 2 and 3 Piping Section III, Division 1

*Inquiry:* What procedure may be used to evaluate the design of hollow circular cross section welded attachments on Classes 2 and 3 pipe under Section III, Division 1?

*Reply:* It is the opinion of the Committee that the procedures listed below may be used to evaluate the design of hollow circular cross section welded attachments on Classes 2 and 3 pipe under Section III, Division 1.

#### **1.0 LIMITATIONS OF APPLICABILITY**

**1.1** The attachment shall be welded to the pipe along the entire circumference by either a full penetration weld, a fillet weld or a partial penetration weld.

**1.2** The attachment material and pipe material shall have essentially the same moduli of elasticity and coefficients of thermal expansion.

**1.3** The constants, defined in Section 2.0, fall within the following ranges:

(a)  $4.0 \leq \gamma \leq 50.0$ 

(b)  $0.2 \le \tau \le 1.0$ 

(c)  $0.3 \leq \beta \leq 1.0$ , and

(d) the axis of the attachment is perpendicular to the axis of the run pipe.

1.4 The attachment shall be made on straight pipe with the nearest edge of the attachment weld located at a minimum distance of  $\sqrt{RT}$  from any other weld or discontinuity (see Section 2.0 for definitions of  $\overline{R}$ and T). For multiple attachments located at a distance less than  $\sqrt{RT}$  to each other, the stress effects for each individual attachment shall be superimposed.

## 2.0 NOMENCLATURE AND DEFINITIONS (See Fig. 2)

 $\begin{aligned} R_o &= \text{run pipe outside radius, in.} \\ r_o &= \text{attachment outside radius, in.} \\ r_i &= \text{attachment inside radius, in.} \\ T &= \text{nominal run pipe wall thickness, in.} \\ t &= \text{nominal attachment wall thickness, in.} \\ D_o &= \text{outside diameter of the run pipe, in.} \\ \frac{d_o}{R} &= \text{outside diameter of the attachment, in.} \\ \hline{R} &= \text{mean radius of run pipe, in.} \\ \mathcal{A}_T &= \pi \left( r_o^2 - r_i^2 \right) \\ Z_T &= I_T / r_o \\ \hline{I}_T &= \frac{\pi}{4} (r_o^4 - r_i^4) \end{aligned}$ 

$$\overline{A} = \frac{\pi}{2} (r_o^2 - r_i^2)$$
  
$$\overline{J} = \text{lesser of } \pi r_o^2 T \text{ or } Z_T$$

 $\begin{aligned} \gamma &= R_o/T \\ \gamma &= R_o/T \\ \tau &= t/T \\ \beta &= d_o/D_o \\ C &= A_o (2\gamma)^n, \beta^n, \tau^n, \text{ but not less than 1.0} \end{aligned}$ 

The equation for C shall be used to determine  $C_{\mu\nu}$ ,  $C_L$ , and  $C_N$ , based on the following table. Select the maximum value of the pipe and the attachment equations.

Index	Part	β Range	A <sub>a</sub>	$n_1$	$n_2$	n <sub>3</sub>
C.,.	Pipe	0.3-1.0	1.40	0.81	(a)	1.33
'n	attachment	0.3-1.0	4.00	0.55	(b)	1.00
С.	Pipe	0.3-1.0	0.46	0.60	-0.04	0.86
- 1.	attachment	0.3-1.0	1.10	0.23	-0.38	0.38
С.,	Pipe	0.3-0.55	0.51	1.01	0.79	0.89
- /4	attachment	0.3-0.55	0.84	0.85	0.80	0.54
	Pipe	> 0.55-1.0	0.23	1.01	- 0.62	0.89
	attachment	> 0.55-1.0	0.44	0.85	-0.28	0.54

NOTES:

(a) Replace  $\beta n^2$  with  $e^{-1.2\beta 3}$ . (b) Replace  $\beta''_2$  with  $e^{-1.35\beta 3}$ .

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CASE (continued)

CASES OF ASME BOILER AND PRESSURE VESSEL CODE

- $C_T = 1.0$  for  $\beta \leq 0.55$
- $C_T = C_N$  for  $\beta = 1.0$ , but not less than 1.0;  $C_T$ should be linearly interpolated for  $0.55 < \beta$ < 1.0, but not less than 1.0
- $B_{\nu} = 0.5 \ (C_{\nu})$ , but not less than 1.0
- $B_L = 0.5 (C_L)$ , but not less than 1.0
- $B_N = 0.5 (C_N)$ , but not less than 1.0
- $B_T = 0.5 (C_T)$ , but not less than 1.0
- $K_T = 2.0$  for fillet or partial penetration welds = 1.8 for full penetration welds
- $S_{h}$  = basic material allowable stress at maximum (hot) temperature, psi (lesser of attachment or pipe material allowable)
- $S_c$  = basic material allowable stress at ambient temperature, psi (lesser of attachment or pipe material allowable)
- $S_d = f (1.25S_c + 0.25S_h)$ , psi, as defined in NC/ ND-3611.2 (lesser of attachment or pipe material allowable)
- $S_{\gamma}$  = yield stress at temperature, psi (lesser of attachment or pipe material yield stress)
- $Z_{wn}$  = section modulus of fillet weld or partial penetration weld about the neutral axis of bending parallel to run pipe center line, in.3
- $Z_{wl}$  = section modulus of fillet weld or partial penetration weld about the neutral axis normal to the run pipe center line, in.3
- $Z_{wr}$  = torsional section modulus of fillet weld or partial penetration weld for torsional loading, in.3
- $A_{w}$  = fillet weld or partial penetration weld throat area, in.2
- $M_L$  = bending moment applied to the attachment as shown in Fig. 1, in.-lb
- $M_N$  = bending moment applied to the attachment as shown in Fig. 1, in.-lb
- $M_T$  = torsional moment applied to the attachment as shown in Fig. 1, in.-lb
- $Q_1$  = shear load applied to the attachment as shown in Fig. 1, lb
- $Q_2$  = shear load applied to the attachment as shown in Fig. 1, lb
- W = thrust load applied to the attachment as shown in Fig. 1, lb

 $M_L$ ,  $M_N$ ,  $M_T$ ,  $Q_1$ ,  $Q_2$ , and W are determined at the surface of the pipe. The values of attachment loads used in the stress evaluation (Section 3.0) are based on the loads used in the different Code equations.  $M_L^{**}, M_N^{**}, M_T^{**}, Q_1^{**}, Q_2^{**}$ , and  $W^{**}$  are absolute values of maximum loads occurring simultaneously under all service loading conditions.

SUPP. 11 - NC

#### 3.0 EVALUATION PROCEDURE

The loads on the attachment cause stresses in the pipe wall. Equations are provided in 3.1(a) to determine these stresses. The attachment stresses are then added to the piping system stresses at the attachment. The piping system stresses are determined by NC-3652 Eq. (8), NC-3653.1 Eq. (9), and NC-3653.2 Eqs. (10), (10a), and (11) for straight pipe. The Code equations, including the attachment stress terms are given in 3.1(b). The attachment stresses  $S_{MT}$ ,  $S_{NT}$ , and  $S_{PT}$  are to be calculated for the loading conditions corresponding to NC-3652 Eq. (8), NC-3653.1 Eq. (9), and NC-3653.2 Eqs. (10), (10a), and (11). For example, in calculating  $S_{MT}$  for use in NC-3652 Eq. (8),  $W, M_L, M_N, Q_1, Q_2$ , and  $M_T$  are the loads on the attachment due to weight and other sustained loads.

There are additional equations given in 3.1(c) and 3.2(b), for fillet welded or partial penetration weld atttachments, that also must be checked for attachment stresses. These are based on the absolute values of maximum loads occurring simultaneously under all service loading conditions.

#### 3.1 Analysis of Attachment Welded to Pipe with a **Full Penetration Weld**

(a) Calculate the stresses  $S_{MD}$   $S_{ND}$   $S_{PT}$ , and  $S_{NT}^{**}$ :

$$S_{MT} = \frac{B_{W}W}{A_{T}} + \frac{B_{N}M_{N}}{Z_{T}} + \frac{B_{L}M_{L}}{Z_{T}} + \frac{Q_{1}}{\overline{A}} + \frac{Q_{2}}{\overline{A}} + \frac{B_{T}M_{T}}{\overline{J}} \quad (1)$$

$$S_{NT} = \frac{C_{W}W}{A_{T}} + \frac{C_{N}M_{N}}{Z_{T}} + \frac{C_{L}M_{L}}{Z_{T}} + \frac{Q_{1}}{A} + \frac{Q_{2}}{A} + \frac{C_{T}M_{T}}{T} \quad (2)$$
$$S_{PT} = K_{T}(S_{NT}) \quad (3)$$

$$= K_T(S_{NT}) \tag{3}$$

$$S_{NT}^{**} = \frac{C_{II} \mathcal{W}^{**}}{A_{T}} + \frac{C_{N} M_{N}^{**}}{Z_{T}} + \frac{C_{L} M_{L}^{**}}{Z_{T}} + \frac{Q_{1}^{**}}{\overline{A}} + \frac{Q_{2}^{**}}{\overline{A}} + \frac{C_{T} M_{T}^{**}}{\overline{T}}$$
(4)

(b) The following modified Code equations shall be satisfied.

(1) NC-3652 Eq. (8) becomes:

 $S_{SL} = B_1 \frac{PD_o}{2t_n} + B_2 \frac{M_A}{Z} + S_{MT} \le 1.5S_h$  (NB-8) where  $B_1 = 0.5$  and  $B_2 = 1.0$  for straight pipe.

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**CASE** (continued)

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Ε

#### CASES OF ASME BOILER AND PRESSURE VESSEL CODE

(2) NC-3653.1 Eq. (9) becomes:

 $S_{OL} = B_1 \frac{P_{\max} D_o}{2t_n}$ +  $B_2 \frac{M_A + M_B}{Z} + S_{MT}$   $\leq 1.8S_h$  but not greater than 1.5S<sub>y</sub> for Level A and B loadings  $\leq 2.25S_h$  but not greater than 1.8S<sub>y</sub> for Level C loadings  $\leq 3.0S_h$  but not greater than 2.0S<sub>y</sub> for Level D loadings

(3) NC-3653.2 Eq. (10) becomes:

$$S_E = \frac{iM_C}{Z} + \frac{S_{PT}}{2} \le S_A \tag{NC-10}$$

(4) NC-3653.2 Eq. (10a) becomes:

$$\frac{iM_D}{Z} + \frac{S_{PT}}{2} \le 3.0 \ S_c$$
 (NC-10a)

(5) NC-3653 2 Eq. (11) becomes:

$$S_{TE} = \frac{PD_o}{4t_n} + 0.75i\left(\frac{M_A}{Z}\right) + i\left(\frac{M_c}{Z}\right) + S_{MT} + \frac{S_{PT}}{2} \le (S_h + S_A) \quad (\text{NC-11})$$

where all terms except attachment stresses are defined in NC-3652. In Eq. (NC-11),  $S_{MT}$  is the same as used in Eq. (NC-8), and  $S_{PT}$  is the same as used in Eq. (NC-10). (c) In addition to the Code equations, the following equations shall also be satisfied:

$$S_{NT}^{**} \le 2S_{v} \tag{5}$$

$$\frac{Q_1^{**}}{\overline{A}} + \frac{Q_2^{**}}{\overline{A}} + \frac{M_T^{**}}{\overline{J}} \le S_y \tag{6}$$

#### 3.2 Analysis of Attachment Welded to Pipe With Fillet Welds or Partial Penetration Welds

(a) The requirements of 3.1 shall be met.

(b) The following additional requirements shall be met:

$$\frac{W^{**}}{A_{w}} + \frac{M_{L}^{**}}{Z_{wl}} + \frac{M_{N}^{**}}{Z_{wn}} + \frac{[(Q_{1}^{**})^{2} + (Q_{2}^{**})^{2}]^{1/2}}{A_{w}} + \frac{M_{T}^{**}}{Z_{wl}} \leq 2S_{y} \quad (7)$$

$$\left\{ \left(\frac{W^{**}}{A_{w}}\right)^{2} + 4 \left[ \left(\frac{Q_{1}^{**} + Q_{2}^{**}}{A_{w}}\right) + \frac{M_{T}^{**}}{Z_{wl}}\right]^{2} \right\}^{1/2} \leq S_{y} \quad (8)$$

#### 3.3 Analysis

Analysis complying with this Case shall be included in the Design Report for the piping system.

#### 4.0 IDENTIFICATION

This Case number shall be shown on the Data Report Form.

NOTE: The potential for increased stress at the attachment welds, which may occur as a result of differential metal temperatures between the attachment and the run, should be considered in the design evaluation.

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CASE (continued)

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CASES OF ASME BOILER AND PRESSURE VESSEL CODE





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# **C** TEST DATA AND RESULTS

## INTRODUCTION

The description of the testing is contained in Section 3. Table 3-1 contains a summary of the results. This Appendix contains more details regarding the test data. For each of the 4 tests (A, B, C, D) the following is provided:

1. Standard sheets containing load-displacement data.

Data sheets are provided for the two loading directions: "Positive Loading" and "Negative Loading", (e.g. up and down). Both the "Loading" and "Unloading" conditions for each of the directons are included for a total of four data sheets. The sheets are used to determine the linear slopes of the load-deflection curves for the four loading conditions.

The data includes loads, deflections etc. The columns identified as "Modified" are for the case where adjustments are required to the data, such as resetting a dial gauge, etc. No modifications were required for these tests.

2. Summary Load Deflection Sheet.

This sheet contains a plot of the load deflection curve and the four straight lines determined from the load-displacement data (item 1 above). This plot indicates the reasonableness of the slope of the load deflection curves.

3. Fatigue Test Data Analysis.

This sheet contains the displacements and number of cycles at each displacement (if more than one displacement is used). The calculations of the SIF are included. The stiffness used in the calculation is the average value of the four loading conditions (in item 1). Equivalent cycles are also calculated.

Test Data and Results

## TEST DATA

# SPECIMEN: TRUNNION ON PIPE

## TEST IDENTIFICATION: A





<u>е</u> 4 5

15

P2 2 2 2

C-4

F = Fo + m 8 TEST #:

ю

DATA POINT



	IG CONDITION		200		-0.800 -0.600 -0.400 -0.200 0.400		200		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~				1600							- 5500			0000		(INCHES)				
FATIGUE - LOAD DEFLECTION CURVE	NEGATIVE LOAD - UNLOADIN				-1.800 -1.500 -1.400 -1.200 -1.000			~	(1	sa					7										DEFLECTION			TEST LOAD-DEFLECTION	
TYPE:	খো	3.215	NOMINAL	(ISI)	-34.5	-30.5	-27.1	-24.01	-19.61	-14.3	-11.1	9.7-	4.0	0.0															
	<u>6</u> Fo (LBS) =	= ( <sub>t</sub> NI) =	BASED	ON "m"	-24071	-21281	-1889	-16761	1969[1-	-1001	1222-	-5491	-300	2	_					_									
	TS, N =	<u>46.0625</u>	SLOPE FOR START TO	DATA POINT (LBS/INCH)	1826	1794	17631	1736	1697	16501	16081	15521	14041	N/AI															_
	ATA POIN 1650	r (IN) ≈	DLOAD	u (SBJ)	-2650	-23001	-19981	-17491	-1397	-9941	-753	-5181	-2551	21						-									
ION-PIPE-A	E ASED ON N D IE OF "m" =	Ŀ	MODIFIE	δ (NCHES)	-1.726	-1.557	-1.412	-1.283	-1.097	-0.874	-0.738	-0.6001	-0.449	-0.2661						_	_								
TRUNN	"m" TO BE	⟨SI, M=F x	ED LOAD	ES)	-26501	-2300	-1998	-1749	-1397	-9941	-7531	-5181	-2551	21		-													
Ŧ	ŶE	L STRESS = MZ	MEASUR	S (INCHES)	-1.726	-1.557	-1.412	-1.283	1760.1-	-0.874	-0.738	-0.600	-0.4491	-0.266															
TEST	+ 0 L 11 11	NOMINA	DATA	**	-	2	6	4	s	ø	1	•0	σ	đ	5	5	ü	4	15	16	17	8	5	20	21	22	53	24	25

NOTES: Deflection is in an upward direction.



Test Data and Results

### FATIGUE TEST DATA ANALYSIS

## TEST #: TRUNNION-PIPE-A

COMPONENT: TRUNNION ON PIPE

STIFFNESS (lbs/in) = 1741

MOMENT ARM (in)= <u>46.0625</u>

D (in) = <u>4.5</u>

t (in) = <u>0.237</u>

Z (in<sup>3</sup>) = 0.0982(D<sup>4</sup>-(D-21)<sup>4</sup>)/D= 3.215

TEST DISPLACEMENT/CYCLE DATA:

CONDITION	DISPLACEMENT	EFFECTIVE	NOMINAL	NUMBER
#	AMPLITÜDE	APPLIED	STRESS	OF TEST
	(+/-) (in.)	LOAD (lbs)	(+/-) (psi)	CYCLES
	d <sub>i</sub>		S	NI
1	1.05	1828	26,193	500
2	0.00	0	Ö	Ō
3	0.00	0	0	0
4	0.00	0	0	Ō
5	0.00	0	0	ō
6	0.00	0	0	Ō
7	0.00	Ũ	Ō	Ō
8	0.00	Ō	Ō	Ō
TOTAL CYC			TOTAL CYCLES:	500

THE EQUIVALENT NO OF CYCLES, BASED ON A DISPLACEMENT OF  $d_{max} =$ 

IS:  $N_{eq} = SUM(d_1/d_{max})^5 * N_1 = 500$ 

FOR MEASURED DIMENSIONS:

 $i = 245,000 * N_{eq}^{(-0.2)}/S = 2.70$ 

FOR NOMINAL DIMENSIONS:  $Z(IN^3) = 3.215$ 

i = 2.70

1.05

COMMENTS:

- 1. HT # 0950276C (PIPE), #64022A (TRUNNION)
- 2. Load cell data indicated loads initially about +/- 2000 lbs.
- 3. Failure occured at toe of weld on pipe at bottom of trunnion.
- 4. Crack grew around bottom of weld, approximately 1/2 around trunnion.
## TEST DATA

# **SPECIMEN:** <u>TRUNNION ON PIPE</u>

# TEST IDENTIFICATION: **B**

10 E		m* TO 8	IE BASED ON N	DATA PO	INTS, N =	শ			£	OSTITVE	[- GYOT	OADIN	G CONDITIO	7
		THE VML	- H, JO 30	1971			a							
NML STI	7W = 8534	KSI, M	FxL,	-010	52.5	= ("N)Z	3215							
	MEASURE		MODIEL MODIEL	0.01	SLOPE FOR	PASED F	NOMINAL	4	000	-2.500	-2.000	-	0.1.000	000 0050
	LECI ON	1	vertection x	3.	DATA PONT	, m, NO				-	-			
é	CHEB)	(JBS)	(INCHES)	(188)	(LBS/INCH)	(185)	(424)						Y	
	0	10	0.0001	0	NIN	0				_	-			- 1000
-	-0.26	-514.5(	-0.2601	-514	1,979	181-	(9.6)							
	0.5331	19001-	10030-	10001-	1,885	-968	(13.6)			_		ł	-	1630
	-0.8551	-1540	-0,855	-1540	1,797	1531-	21.77				1		2	
	-1.1151	12121-	-1.115	12161-	1.716	-2004	(28.4)				ł		1	
	-1.471	-2360(	-1,470	-23601	1.610	-2642	(17.4)	(sc			1	ľ		0002
	1.584	-2477	1.5841	-2477	1,554	-2847	(40.3)	111		4				
	-1.351	-27261	1058.1-	-27261	1.479	-3325	(47.0)	NO		-		Y		2800
	-2.1181	-2902	-2.116	-29021	1,388	2002	(53.8)	al		-		Ì		
	-2.346	12500?*	845.5	109007	1,308	4216	(28.7)	av			/	7		
	-2.491	144157	-2.490	114410-	1,247	-4475	(63.3)	01		Ì		ł		000
	-2.576	11217	-2.576	11815-	1,202	4630	(85.5)	1		F				
	-2,5781	-3188	-2.576	-3188	1.174	0091-	(85.5)			_	1			10091
	-2.576	19817	-2,576	-3185	1,158	1630	(65.5)				7			
	-2.5781	-3156	-2,576	13815-	1,143	1830	(65.5)				Ň			1000
	-2.576	12021-	-2.576	12021-	1124	1630	(82.5)			-	-	ľ	_	
-	-2,5781	-32101	-2,576	-32101	1.127	1830	(65.3)			`				
	-2.576	1212-	-2.576	12120-	1,122	-4630	(65.5)			4	-			1500
	-2.578	-3224	-2,576	-3224	1.178	1830	(65.0)							
-				-										2000
											ä	FLECTION	(INCHES)	
-								I.						
									1831	LOND-DEPL	<b>BCTION</b>	N-1	AR-DEFLECTION	



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--- TEST LOAD-DEFLECTION

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5			-			-		4			5														0.400		
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																	K	٩							400		1
E LUA																			$\backslash$						9 00		
7A114.																						<u> </u>			00 -0.6		
VAN						-									-#-							-7		•	00 -0.8		
			L							(	sc	IN	10	н) (	JA(	רס	_1			'					0.1.		
64.119	<u>3.215</u>	NOMINAL	STRESS		(KSI)	6.0	4.2	8.6	12.7	17.0	21.8	26.2	27.7	30.51	32.4	34.2	35.31	36.2									
5 Fo (LBS) = 1	= (ĮNĴ) =	u	BASED	"m" NO	(LBS)	641	294	607	8951	12031	1541	1850	19581	2155	22921	2414	2491	2560									
= ×.	<u>5.5</u>		START TO	DATA POINT	(LBS/INCH)	NA	1,474	1,494	1,503	1,495	1,464	1,425	1,391	1,365	1,345	1,326	1,311	1.295									
A POINTS 1495	= (NL)		0AD	} ; L	(LBS)	64	291	606	8991	11991	15001	1749	1800	1961	2064	21441	2195	22251									
ASED ON N DAT	ц.			δ δ	(INCHES)	-0.936	-0.782	-0.573	-0.380	-0.174	0.052	0.2591	0.331	0.4631	0.555	0.6361	0.6381	0.734	_								
'm" TO BE B THE VALUE	(SI, M≓F x L,			ן 	(LBS)	64.119	291.17	606.111	180.081	1199.3731	1499.666	1748.639	1799.9591	1961.092	2063.631	2144.197	2195.467	2224.763									
ъ С	. STRESS = MZ H		MEASURI		(INCHES)	-0.936	-0.782	-0.5731	-0.38	-0.174	0.0521	0.2591	0.3311	0.4631	0.5551	0.6361	0.6881	0.7341									
+ 01 = 1	NOMINAL		DATA	z	14	-	7	m	4	5	G	F	80	6	ē	=	12	51	4	15	9	17	18	6	20	21	55

NOTES:

3 2 52

22

FATIGUE - LOAD DEFLECTION CURVE

TYPE:

VIC VIC I NECATIVE LOAD

NOLLINOU DI

# LS C-12

TRUNNION-PIPE-B





SUMMARY LOAD-DEFLECTION

Test Data and Results

(вайноя) ахол

### FATIGUE TEST DATA ANALYSIS

### TEST #: TRUNNION-PIPE-B

COMPONENT: TRUNNION ON PIPE

STIFFNESS (lbs/in) = 1732

MOMENT ARM (in)= <u>45.5</u>

D (in) = <u>4.5</u>

t (in) = <u>0.237</u>

Z (in<sup>3</sup>) = 0.0982(D<sup>4</sup>-(D-21)<sup>4</sup>)/D= 3.215

TEST DISPLACEMENT/CYCLE DATA:

CONDITION	DISPLACEMENT	EFFECTIVE	NOMINAL	NUMBER
#	AMPLITÙDE	APPLIED	STRESS	OF TEST
	(+/-) (in.)	LOAD (lbs)	(+/-) (psi)	CYCLES
	d,		S	NI
1	0.90	1559	22,057	1,280
Ž	0.00	Ö	Ō	Ö
3	0.00	Ō	Ō	Ō
4	0.00	0	Õ	Ō
5	0.00	Ō	Ö	Ö
6	0.00	Ō	Ō	Ō
7	0.00	ō	Ō	Ō
8	0.00	Ő	Ö	Ō
			TOTAL CYCLES:	1,280

THE EQUIVALENT NUMBER OF CYCLES, BASED ON A DISPLACEMENT O d<sub>max</sub> = 0.9

IS:  $N_{eq} = SUM(d_i/d_{max})^5 * N_i = 1,280$ 

FOR MEASURED DIMENSIONS:

 $i = 245,000 * N_{eq}^{(-0.2)}/S = 2.656$ 

FOR NOMINAL DIMENSIONS:  $Z(IN^3) = 3.215$ 

i = 2.656

COMMENTS:

1. HT # 0950276C (PIPE), #64022A (TRUNNION)

2. Load initially in up direction.

### TEST DATA

# **SPECIMEN: TRUNNION ON PIPE**

# TEST IDENTIFICATION: $\underline{C}$









NOTES: Deflection is in an upward direction.





SUMMARY LOAD-DEFLECTION

### FATIGUE TEST DATA ANALYSIS

### TEST #: TRUNNION-PIPE-C

COMPONENT: TRUNNION ON PIPE

STIFFNESS (lbs/in) = <u>1702</u>

MOMENT ARM (in)= <u>46.125</u>

D (in) = <u>4.5</u>

t (in) = <u>0.237</u>

 $Z (in^3) = 0.0982(D^4 - (D-21)^4)/D = 3.215$ 

TEST DISPLACEMENT/CYCLE DATA:

CONDITION	DISPLACEMENT	EFFECTIVE	NOMINAL	NUMBER
#	AMPLITUDE	APPLIED	STRESS	OF TEST
	(+/-) (in.)	LOAD (lbs)	(+/-) (psi)	CYCLES
	di		S	Ni
1	0,90	1532	21,978	934
2	0.00	Ō	Ō	Ō
3	0.00	Ō	Ō	ō
4	0.00	0	0	Ō
5	0.00	Ō	Ö	Ō
6	0.00	0	Ō	Ō
7	0.00	Ö	0	Ō
8	0.00	Ö	Ō	Ō
L			TOTAL CYCLES:	934

THE EQUIVALENT NUMBER OF CYCLES, BASED ON A DISPLACEMENT O  $d_{max} = 0.9$ 

IS:  $N_{eq} = SUM(d_1/d_{max})^5 * N_1 = 934$ 

FOR MEASURED DIMENSIONS:

 $i = 245,000 * N_{eq}^{(-0.2)}/S = 2.839$ 

FOR NOMINAL DIMENSIONS:  $Z(IN^3) = 3.215$ 

i = 2.839

COMMENTS:

1. HT # 0950276C (PIPE), #64022A (TRUNNION)

2. Load initially in up direction.

### TEST DATA

# **SPECIMEN: TRUNNION ON PIPE**

# TEST IDENTIFICATION: D

**EPRI Licensed Material** 



Loading is in an upward direction. For purposes of the evaluation of the stiffness calculation, the directions of loading (e.g. positive load-loading condition) will remain the same. The sign of the deflection will indicate the load direction.



FATIGUE - LOAD DEFLECTION CURVE

TYPE:

TRUNNION-PIPE-D

TEST #:

.



NOTES:







FATIGUE TEST DATA ANALYSIS

### TEST #: TRUNNION-PIPE-D

COMPONENT: TRUNNION ON PIPE

STIFFNESS (lbs/in) = <u>1841</u>

D (in) = <u>4.5</u>

MOMENT ARM (in)= <u>45.5</u>

 $Z (in^3) = 0.0982(D^4 - (D - 21)^4)/D = 3.215$ 

TEST DISPLACEMENT/CYCLE DATA:

CONDITION	DISPLACEMENT	EFFECTIVE	NOMINAL	NUMBER
#	AMPLITUDE	APPLIED	STRESS	OF TEST
	(+/-) (in.)	LOAD (lbs)	(+/-) (psi)	CYCLES
	ďi		S	Ni
1	0.80	1473	20,845	1,866
2	0.00	ō	Ō	Ō
3	0.00	Ō	Ō	ō
4	0.00	ō	Ō	Ō
5	0.00	Ō	Ö	Õ
6	0.00	Ö	Ō	Ō
7	0.00	Ō	Õ	Ō
8	0.00	Ö	Ō	Ō
			TOTAL CYCLES:	1,866

t (in) = <u>0.237</u>

THE EQUIVALENT NUMBER OF CYCLES, BASED ON A DISPLACEMENT O  $d_{max} = 0.8$ 

IS:  $N_{eq} = SUM(d_1/d_{max})^5 * N_1 = 1,866$ 

FOR MEASURED DIMENSIONS:

 $i = 245,000 * N_{eq}^{(-0.2)}/S = 2.606$ 

FOR NOMINAL DIMENSIONS:  $Z(IN^3) = 3.215$ 

i = 2.606

COMMENTS:

1. HT # 0950276C (PIPE), #64022A (TRUNNION)

2. Load initially in up direction.



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Program:

TR-110162

Nuclear Power

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