

# Directional Stress Indices and Stress Intensification Factors for 90° Elbows (PWRMRP-06)



PWR Materials Reliability Project (PWRMRP)

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PWR Materials Reliability Project (PWRMRP)

TR-113889

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# **REPORT SUMMARY**

This report provides equations, based on analyses and test data, for determining the directional stress indices and stress intensification factors (SIFs) for 90° elbows. Present methodologies used to determine these parameters are generally overly conservative. The report contains results of an investigation into the stress intensification factors and directional stress indices of 90° elbows.

#### Background

Fatigue is a major concern in the design and engineering of piping systems. The ASME Section III Code and ANSI B31 piping design codes use factors such as stress indices and stress intensification factors to account for fatigue effects produced by combined loading and moments.

#### Objectives

- To determine if the use of directional stress indices and SIFs is appropriate
- To establish a methodology for evaluating directionality effects

#### Approach

A review of the present approach for the evaluation of 90° elbows in accordance with the ASME and B31 piping design codes provided an understanding of the current methodology. Component tests and finite element analyses (FEAs) were performed on representative elbow configurations. Various methods of combining moments were performed. Results were compared to those generated by the FEAs.

#### Results

A more accurate method for combining moments was developed and is based on a modified version of the square root sum of the squares (SRSS) approach. Based on the models and loading conditions in this study, it was determined that the expression accounting for directionality in ASME Code Case N-319-2 yielded conservative results as high as 29% according to the FEA results. In contrast, the method used by ANSI B31.3 yielded non-conservative results as high as 49% when compared to FEA.

#### **EPRI** Perspective

Design for fatigue is a major concern for any power or process facility. Accurate methods of engineering for fatigue are important for cost-effective design, for root cause failures, and for evaluating remaining fatigue life of plant designs. The work being done under EPRI's SIF optimization program continues to establish the technical justification to allow for reductions in current Code stress intensification factors. The results of this program can provide a basis to reduce the scope of ongoing pressure boundary component testing inspection programs in

operating nuclear power plants. Examples include reduction in the inspection scope of postulated high- and moderate-energy line break locations and reduction of snubber testing.

#### TR-113889

#### Keywords

ASME Code Fatigue Piping design and analysis Stress intensification factors Stress indices

## ABSTRACT

This report was prepared under the auspices of the EPRI project on Stress Intensification Factor Optimization. The behavior and fatigue life of elbows is a major consideration in the design and evaluation of piping systems. This report presents the results of an investigation of the directional stress intensification for 90° elbows. The investigation included a literature survey, testing program, finite element analysis, comparison of analysis to test results, and recommendations for removing conservatism in evaluating elbows.

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# **1** INTRODUCTION

The objective of this study was to investigate methodologies and determine if the use of directional stress indices and SIFs are appropriate. Finite element analyses were conducted as a part of this study to develop a moment combination methodology appropriate for elbows. Test data for combined loading serve as a basis for establishing a methodology for evaluating directionality effects.

## 1.1 Nomenclature

Figure 1-1 shows the configuration and applied moments for the evaluation of stress indices and stress intensification factors for  $90^{\circ}$  elbows. Table 1-1 lists nomenclature used in this report.



Figure 1-1 90° Elbow Configuration

#### Introduction

#### Table 1-1 Nomenclature

Term	Description
a, b, n	constants used in SQ-SUM(ADJUSTED) methodology to represent stress intensities
A, B, C	constants used to represent load cases, for example, $M_i = A M$ , $M_o = B M$ , $M_i = C M$
D <sub>o</sub>	outside diameter of the elbow or attached pipe
Е	Young's modulus
B31.3-EQ17	stress intensity calculated using a method corresponding to the approach used by B31.3 (eq. 2-7)
h	characteristic of the elbow, $h=(t_n/r)(R/r)=t_nR/r^2$
i	stress intensification factor for in-plane bending moments
i <sub>o</sub>	stress intensification factor for out-of-plane bending moments
i,	stress intensification factor for torsion moments
G	Bulk modulus
L <sub>1</sub> , L <sub>2</sub>	attached pipe tangent lengths, in.
М	moment that produces a nominal stress of 10 ksi in the pipe
$\mathbf{M}_{_{i1,}} \mathbf{M}_{_{i2}}$	in-plane bending moments, in-lb
$\mathbf{M}_{_{\mathrm{o}1}},\mathbf{M}_{_{\mathrm{o}2}}$	out-of-plane bending moments, in-lb
<b>M</b> <sub>t1</sub> , <b>M</b> <sub>t2</sub>	torsion moments, in-lb
r	mean radius of the elbow or attached pipe, $r = (D_o - t_n)/2$ , in.
R	bend radius of the elbow, in.
SQ-SUM	stress intensity calculated using a method which adds torsion and out-of-plane bending effects directly (eq. 2-10)
SQ-SUM (ADJUSTED)	stress intensity using a modified SQ-SUM methodology (eq. 2-12)
SRSS	stress intensity calculated using the "square root-sum of the squares methodology" (eq. 2-6)
$S_{1}, S_{2}, S_{3}$	stress intensity due to M applied to the pipe as in-plane, out-of-plane and torsion respectively
t <sub>n</sub>	wall thickness of the elbow or attached pipe, in.
Z	section modulus, $(in^3) = 0.0982(D_o^4 - (D_o^2 - 2t_n)^4)/D_o$
$r^2$	correlation factor squared
ν	Poisson's ratio

#### 1.2 Background

The objective of this study was to investigate methodologies and determine if the use of directional stress indices and SIFs are appropriate. Finite element analyses were conducted as a part of this study to develop a moment combination methodology appropriate for elbows. Test data for combined loading serves as a basis for establishing a methodology for evaluating directionality effects.

At present, the ASME Section III Code [1] defines the SIF for an elbow by the equation:

$$i = 0.9/h^{2/3}$$
 (equation 1-1)

where h is defined as the elbow "flexibility characteristic" given by:

$$h = t_{a}R/r^{2}$$
 (equation 1-2)

where

 $t_n$  = the nominal wall thickness of the elbow (or attached pipe)

R = the bend radius of the elbow or pipe bend

r = the mean radius of the elbow or attached pipe

Stresses at a point j are calculated using the resultant moment:

 $\mathbf{M}_{i} = [\mathbf{M}_{xi}^{2} + \mathbf{M}_{yi}^{2} + \mathbf{M}_{zi}^{2}]^{1/2}$  (equation 1-3)

and the expression:

$$S = iM/Z$$
 (equation 1-4)

where

M is given by M<sub>i</sub>.

 $M_{xi}$ ,  $M_{yi}$ , and  $M_{zi}$  are the moments about the x, y, and z axes at point j.

Z is the section modulus of the attached pipe.

The assumption, which is the focus of this study, is that the SIF given by eq. 1-1 is applicable for all moments (for example, in-plane, out-of-plane, and torsion for the elbow). ANSI B31.1 [2] follows the same approach. However, ANSI B31.3 [3] uses a different methodology that takes into account the directionality of the loading. B31.3 defines SIFs for in-plane loading,  $i_i$  which is the same as eq. 1-1. However, for out-of-plane loading, the SIF is defined as:

$i_0 = 0.75/h^{2/3}$	(equation 1-5)
----------------------	----------------

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In B31.3, the stresses used to evaluate thermal expansion are calculated using equation 17 in *Process Piping*, *B31.3*, *Power Piping* [3]:

$$S_{\rm F} = [S_{\rm h}^2 + 4S_{\rm t}^2]^{1/2}$$
 (equation 1-6)

where  $S_{b}$  is the resultant bending stress given by:

$$S_{b} = [(i_{1}M_{i})^{2} + (i_{0}M_{o})^{2}]^{0.5}/Z$$
 (equation 1-7)

where

 $M_i$  = the in-plane bending moment

 $M_0$  = the out-of-plane bending moment

 $S_t$  = the torsional stress given by M/2Z where  $M_t$  is the torsional moment

This approach is clearly less conservative than the approach followed by ASME Section III.

The objective of this study is to investigate these methodologies and to determine if the directional SIFs are applicable. This study is limited to 90° bends or elbows with a length of straight pipe welded to both ends such that end effects are precluded (generally considered to be four or more pipe diameters). More significant end effects exist when a flange or another component is attached to the elbow such that the deformation of the elbow is restrained.

## 1.3 Symmetry

Figure 1-1 shows that, for a 90° elbow, there is a transformation of the out-of-plane and torsional moments at the opposite ends of the elbow. Torsion on one end is resisted by an equal (in magnitude) out-of-plane bending moment at the other end. Out-of-plane bending is resisted by an equal-in-magnitude torsional moment at the other end. In-plane bending is resisted by an equal (but opposite in sign) in-plane bending at the other end.

This is true only for 90° elbows or bends. The response (for example, stresses and deflections) of the elbow to moment  $M_0$  at one is the same as the response to the moment  $M_1$  at the other. Thus, a 90° elbow has two characteristic response behaviors, not three. The stress intensification factors for out-of-plane bending and torsion should be the same.

## 1.4 Literature Summary

Many investigations of the behavior of elbows have been reported in the literature. Dodge, Moore, and Rodabaugh [4, 5] discuss a number of these investigations. Markl in 1952 reported the results of tests of various elbows for both in-plane and out-of plane, loading [6]. The tests were performed on a deflection controlled, bending type machine. Various wall thicknesses and bend radii were used in the testing. Markl correlated the test data with the theory proposed by

Introduction

Beskin [7] and others. Based upon this, he proposed the following expressions for estimating the i-factors for in-plane and out-of-plane bending of an elbow:

$$i_i = 0.9/h^{2/3}$$
 (equation 1-8)  
 $i_0 = 0.75/h^{2/3}$  (equation 1-9)

These expressions are the same as those used today by many of the piping design codes.

Markl also noted that the test data SIFs corresponded to about one-half the theoretical stress. This is in line with the ASME Section III position that the SIF, i, is given by:

 $i = C_2 K_2/2$  (equation 1-10)

where

 $C_2$  = the primary plus secondary stress intensity factor

 $K_{2}$  = the peak stress intensity factor

Code Case N-319-2, provides guidance for evaluating elbows in Class 1 piping considering the directionality of the loads [8]. This approach takes into account the directionality of the loadings and provides separate stress indices for in-plane and out-of-plane/torsional loadings. These are given by:

Out-of-plane/torsion: 
$$C_{2x} = 1.71/h^{0.56}$$
 (equation 1-11)

In-plane:  $C_{2z} = 1.95/h^{2/3}$  (equation 1-12)

Code Case N-319-2 suggests that to calculate stresses for combination of moments, the following equation be used:

$$[(C_{2x}M_{x})^{2} + (C_{2y}M_{y})^{2} + (C_{2z}M_{z})^{2}]^{1/2}$$
 (equation 1-13)

where

X = out-of-plane loading

Y = torsion

Z = in-plane loading

Equation 1-13 is permitted to be used only if the flexibility factors listed in the Code Case are used in piping system analyses. See Section 2.10 for a discussion of flexibility factors for selected FEA models.

# **2** FINITE ELEMENT ANALYSIS INVESTIGATION

## 2.1 Objectives

An elbow lends itself to straightforward analysis by the finite element method. An elbow has no discontinuities and can be modeled with a uniform size finite element mesh. Finite element analyses (FEA) were conducted as a part of this study to develop a moment combination methodology appropriate for elbows. In addition, FEA was performed to investigate elbow flexibility.

## 2.2 Models

The parameters of the FEA models are shown in Table 2-1. The basic model had an outside diameter of 4.5 in. The thickness was varied in order to investigate the effects of varying  $D_o/t_n$  values. The bend radius varied from 4.0 to 18 in.

#### Table 2-1 **Summary of FEA Models**

								-Plane Mome	nt	Out-of-Plane Moment			
								C <sub>2</sub> =			C <sub>2</sub> =		
Model	D。	t <sub>n</sub>	D <sub>o</sub> /t <sub>n</sub>	R	r	h	FEA	1.95/h <sup>2/3</sup>	% DIF	FEA	1.71/h <sup>0.56</sup>	% DIF	
							S1			S2=S3			
1	4.5	0.2370	19.0	6.0	2.13	0.313	4.11	4.23	2.84	2.88	3.28	12.12	
2	4.5	0.0446	101.0	6.0	2.23	0.054	13.55	13.67	0.90	7.75	8.78	11.73	
3	4.5	0.2370	19.0	4.0	2.13	0.209	5.17	5.54	6.73	3.48	4.11	15.38	
4	4.5	0.2370	19.0	12.0	2.13	0.626	2.51	2.66	5.81	2.03	2.22	8.68	
5	4.5	0.0446	101.0	4.0	2.23	0.036	17.5	17.92	2.32	10.4	11.02	5.61	
6	4.5	0.1185	38.0	6.0	2.19	0.148	6.91	6.96	0.79	4.33	4.98	13.09	
7	4.5	0.1185	38.0	4.0	2.19	0.099	8.83	9.13	3.25	5.35	6.25	14.43	
8	4.5	0.0446	101.0	18.0	2.23	0.162	6.71	6.57	-2.08	4.81	4.75	-1.36	
9	4.5	0.0446	101.0	12.0	2.23	0.108	8.79	8.61	-2.05	5.87	5.96	1.43	
10	4.5	0.1185	38.0	12.0	2.19	0.296	4.41	4.39	-0.51	3.21	3.38	5.01	
							Мах	imum Value =	6.73	Max	imum Value =	15.38	
	Average Value = 1.80 Average Value =											8.61	

Notes:

1. FEA results are the maximum stress intensity divided by the nominal bending stress in the pipe.

2. All dimensions are in inches.

The expressions for C<sub>2</sub> for in-plane and out-of-plane bending are from Code Case N-319-2.
 This table was produced on an Excel spreadsheet. The number of significant figures is greater than indicated.

All models were based on 90° bends or elbows. The elbows were attached to equal length straight sections of pipe with a length equivalent to five pipe diameters. (This corresponds to  $L_1$  and  $L_2$  of Figure 1-1.) This represents the case where there are no "end effects" that might exist if there were a flange or other component attached that could affect the deflection (ovalization) of the elbow.

A typical finite element model is shown in Figure 2-1. Approximately 3600 shell elements were used in each model. The material properties used in the analyses are E=30,000,000 psi, G=12,000,000 psi, and v=0.28. The finite element analyses were conducted with the COSMOS, version 1.75, software from Structural Research and Analysis Corporation.



#### Figure 2-1 FEA Model

One end of the model was fixed and combinations of three orthogonal moments were applied at the other end of the model. Ten configurations were investigated. Twelve load case combinations were applied to each model. Load combinations were selected using the following procedure:

- 1. The magnitude of the moment was calculated such that the nominal bending stress in a straight pipe would be 10.0 ksi. This moment was defined as "M."
- 2. This moment was applied at point 1 (Figure 1-1) in the three orthogonal directions, that is, inplane, out-of-plane, and torsion, corresponding to M<sub>i</sub>, M<sub>o</sub>, and M<sub>i</sub>. The end of the model is

fixed at point 2. The maximum stress intensity from the FEA results for these three cases is delineated as S1, S2, and S3 respectively for each load case. (S1, S2 and S3 are used in the evaluation of the data as described later.) The maximum stress intensities are divided by the nominal stress intensity in the pipe. S1, S2, and S3 are in the form of stress indices and, thus, are independent of the particular choice of E and M. For example, the same S1 would be obtained by using, for example, E = 10e7 psi and M/Z = 1 psi.

3. A series of load cases were defined where:

$\mathbf{M}_{i} = \mathbf{A} \mathbf{M}$	(equation 2-1)
$\mathbf{M}_{o} = \mathbf{B} \mathbf{M}$	(equation 2-2)
$\mathbf{M}_{t} = \mathbf{C} \mathbf{M}$	(equation 2-3)
$A^2 + B^2 + C^2 = 1.0$	(equation 2-4)

In other words, the magnitude of the applied moment remained constant for all load cases. However, the ratios of in-plane to out-of-plane and torsion were varied.

For each model, five values of the ratio of the torsion moment to the out-of-plane moment (C/B) were selected (0, 0.1, 0.25, 0.5, 1.0). As discussed earlier, the symmetry of a  $90^{\circ}$  elbow or bend is such that out-of-plane bending at one end of an elbow is the same as torsion at the other end. This symmetry will be considered when the results are evaluated.

For each value of C/B, ten values of A/B, the ratio of the in-plane moment to the out-of-plane moment, were selected (0.0, 0.100, 0.250, 0.500, 0.750, 1.000, 1.333, 2.000, 4.000 and 10.000).

As an example, the results of the analyses for model 1 are listed in Tables 2-2 and 2-3 for these values of C/B and A/B. Because of the volume of data, the results of the other models are not listed in as much detail. A summary of the FEA results is provided in Table 2-6 for all models. See Section 2.9 for the significance of Tables 2-4 and 2-5.

Min =A*M		Mon=B*M		M₁ = C*M									
LOAD		In-Plane	Out of-Plane	Torsion	FEA Stress								
CASE	A/B	A	В	С	x104								
LC-1		1	0	0	4.107	= S1							
LC-2		0	1	0	2.884	= S2							
LC-3		0	0	1	2.884	= S3							
COMBINED LOADS, C/B = 0									SQ-SUM				
	A/B	А	В	С	FEA	SRSS	% Dif	SQ-SUM	% Dif	ADJUSTED	% Dif	B31.3-EQ17	% Dif
LC-4	0.1	0.10	1.00	0.00	2.91	2.90	-0.34	2.90	-0.34	2.90	-0.34	2.90	-0.34
LC-5	0.25	0.24	0.97	0.00	2.99	2.97	-0.81	2.97	-0.81	2.97	-0.81	2.97	-0.81
LC-6	0.5	0.45	0.89	0.00	3.20	3.17	-1.01	3.17	-1.01	3.17	-1.01	3.17	-1.01
LC-7	0.75	0.60	0.80	0.00	3.44	3.38	-1.79	3.38	-1.79	3.38	-1.79	3.38	-1.79
LC-8	1	0.71	0.71	0.00	3.63	3.55	-2.23	3.55	-2.23	3.55	-2.23	3.55	-2.23
LC-9	1.333	0.80	0.60	0.00	3.79	3.71	-2.08	3.71	-2.08	3.71	-2.08	3.71	-2.08
LC-10	2	0.89	0.45	0.00	3.97	3.89	-1.94	3.89	-1.94	3.89	-1.94	3.89	-1.94
LC-11	4	0.97	0.24	0.00	4.09	4.05	-1.12	4.05	-1.12	4.05	-1.12	4.05	-1.12
LC-12	10	1.00	0.10	0.00	4.10	4.10	-0.11	4.10	-0.11	4.10	-0.11	4.10	-0.11
						MAX =	-0.11	MAX =	-0.11	MAX =	-0.11	MAX =	-0.11
NOTE:						MIN =	-2.23	MIN =	-2.23	MIN =	-2.23	MIN =	-2.23
COMBINED	LOADS,	C/B = 0.1								SQ-SUM			
	A/B	A	В	С	FEA	SRSS	% Dif	SQ-SUM	% Dif	ADJUSTED	% Dif	B31.3-EQ17	% Dif
LC-4	0.1	0.099	0.990	0.099	3.09	2.90	-6.29	3.17	2.41	3.10	0.20	2.89	-6.69
LC-5	0.25	0.241	0.966	0.097	3.17	2.97	-6.43	3.22	1.47	3.16	-0.54	2.96	-6.79
LC-6	0.5	0.445	0.891	0.089	3.36	3.16	-5.70	3.37	0.33	3.31	-1.22	3.16	-5.97
LC-7	0.75	0.598	0.797	0.080	3.56	3.37	-5.36	3.53	-1.06	3.49	-2.17	3.37	-5.56
LC-8	1	0.705	0.705	0.071	3.71	3.55	-4.40	3.66	-1.30	3.63	-2.11	3.54	-4.54
LC-9	1.333	0.798	0.599	0.060	3.86	3.71	-3.93	3.79	-1.87	3.77	-2.40	3.71	-4.02
LC-10	2	0.894	0.447	0.045	4.00	3.89	-2.65	3.93	-1.59	3.92	-1.86	3.89	-2.70
LC-11	4	0.970	0.242	0.024	4.11	4.04	-1.54	4.06	-1.25	4.05	-1.32	4.04	-1.55
LC-12	10	0.995	0.099	0.010	4.10	4.10	-0.13	4.10	-0.08	4.10	-0.09	4.10	-0.13
						MAX =	-0.13	MAX =	2.41	MAX =	0.20	MAX=	-0.13
						MIN =	-6.43	MIN =	-1.87	MIN =	-2.40	MIN =	-6.79

#### Table 2-2 FEA Summary, Model 1, Combined Loads, C/B = 0 and 0.1

Notes: 1. FEA results are the maximum stress intensity divided by the nominal bending stress in the pipe.2. This table was produced on an Excel spreadsheet. The number of significant figures is greater than indicated.

Table 2-3	
FEA Summary, Model 1 Combined Loads, C/B = 0.25, 0.5, and 1.0	D

COMBINED LOADS, C/B = 0.25									SQ-SUM				
	A/B	А	В	С	FEA	SRSS	% Dif	SQ-SUM	% Dif	ADJUSTED	% Dif	B31.3-EQ17	% Dif
LC-4	0.1	0.097	0.966	0.241	3.31	2.90	-12.45	3.50	5.85	3.37	1.72	2.82	-14.71
LC-5	0.25	0.236	0.943	0.236	3.39	2.97	-12.58	3.53	4.18	3.40	0.38	2.90	-14.63
LC-6	0.5	0.436	0.873	0.218	3.55	3.15	-11.21	3.62	1.95	3.51	-1.08	3.10	-12.78
LC-7	0.75	0.588	0.784	0.196	3.71	3.36	-9.50	3.72	0.25	3.64	-2.03	3.32	-10.64
LC-8	1	0.696	0.696	0.174	3.80	3.53	-7.07	3.81	0.17	3.74	-1.53	3.50	-7.90
LC-9	1.333	0.791	0.593	0.148	3.94	3.70	-6.24	3.89	-1.34	3.84	-2.51	3.68	-6.79
LC-10	2	0.889	0.444	0.111	4.02	3.88	-3.52	3.99	-0.92	3.96	-1.55	3.87	-3.81
LC-11	4	0.968	0.242	0.061	4.13	4.04	-2.12	4.07	-1.39	4.06	-1.56	4.04	-2.20
LC-12	10	0.995	0.099	0.025	4.10	4.10	-0.14	4.10	-0.02	4.10	-0.05	4.10	-0.15
						MAX =	-0.14	MAX =	5.85	MAX =	1.72	MAX=	-0.15
						MIN =	-12.58	MIN =	-1.39	MIN =	-2.51	MIN =	-14.71
COMBINED L	LOADS, C	C/B = 0.5								SQ-SUM			
	A/B	А	В	С	FEA	SRSS	% Dif	SQ-SUM	% Dif	ADJUSTED	% Dif	B31.3-EQ17	% Dif
LC-4	0.1	0.089	0.891	0.445	3.56	2.90	-18.73	3.87	8.65	3.63	1.96	2.63	-26.10
LC-5	0.25	0.218	0.873	0.436	3.63	2.95	-18.63	3.88	6.91	3.65	0.63	2.71	-25.41
LC-6	0.5	0.408	0.816	0.408	3.76	3.12	-17.00	3.91	3.97	3.71	-1.27	2.92	-22.37
LC-7	0.75	0.557	0.743	0.371	3.86	3.31	-14.14	3.94	2.25	3.78	-1.92	3.16	-18.18
LC-8	1	0.667	0.667	0.333	3.95	3.48	-11.81	3.98	0.75	3.85	-2.49	3.36	-14.82
LC-9	1.333	0.766	0.575	0.287	4.01	3.65	-9.03	4.01	-0.10	3.92	-2.44	3.57	-11.12
LC-10	2	0.873	0.436	0.218	4.09	3.85	-5.93	4.05	-1.03	4.00	-2.33	3.81	-7.04
LC-11	4	0.963	0.241	0.120	4.10	4.03	-1.76	4.09	-0.31	4.07	-0.70	4.02	-2.08
LC-12	10	0.994	0.099	0.050	4.10	4.09	-0.22	4.10	0.03	4.10	-0.04	4.09	-0.27
						MAX =	-0.22	MAX =	8.65	MAX =	1.96	MAX=	-0.27
						MIN =	-18.73	MIN =	MIN = -1.03 MIN =		-2.49	MIN =	-26.10
COMBINED L	LOADS, C	C/B = 1.0								SQ-SUM			
	A/B	А	В	С	FEA	SRSS	% Dif	SQ-SUM	% Dif	ADJUSTED	% Dif	B31.3-EQ17	% Dif
LC-4	0.1	0.071	0.705	0.705	3.69	2.89	-21.58	4.08	10.62	3.68	-0.20	2.17	-41.08
LC-5	0.25	0.174	0.696	0.696	3.75	2.93	-21.93	4.08	8.76	3.69	-1.59	2.24	-40.22
LC-6	0.5	0.333	0.667	0.667	3.84	3.04	-20.64	4.08	6.41	3.73	-2.82	2.45	-36.06
LC-7	0.75	0.469	0.625	0.625	3.91	3.19	-18.38	4.08	4.42	3.78	-3.48	2.71	-30.75
LC-8	1	0.577	0.577	0.577	3.98	3.34	-15.99	4.09	2.77	3.83	-3.82	2.95	-25.73
LC-9	1.333	0.686	0.515	0.515	4.04	3.51	-13.13	4.09	1.19	3.89	-3.92	3.23	-20.25
LC-10	2	0.816	0.408	0.408	4.12	3.74	-9.12	4.10	-0.54	3.97	-3.66	3.58	-13.16
LC-11	4	0.943	0.236	0.236	4.16	3.99	-4.03	4.10	-1.28	4.06	-2.30	3.94	-5.26
LC-12	10	0.990	0.099	0.099	4.13	4.09	-0.98	4.11	-0.50	4.10	-0.68	4.08	-1.19
						MAX =	-0.98	MAX =	10.62	MAX =	-0.20	MAX=	-1.19
						MIN =	-21.93	MIN =	-1.28	MIN =	-3.92	MIN =	-41.08

Notes: 1. FEA results are the maximum stress intensity divided by the nominal bending stress in the pipe.

2. This table was produced on an Excel spreadsheet. The number of significant figures is greater than indicated.

Table 2-4	
FEA Summary, Model 1 Combined Loads, C/	/B = 1.5, 2.0, and 4.0

COMBINED LOADS, C/B = 1.5									SQ-SUM				
	A/B	A	В	С	FEA	SRSS	% Dif	SQ-SUM	% Dif	ADJUSTED	% Dif	B31.3-EQ17	% Dif
LC-4	0.1	0.055	0.554	0.831	3.62	2.89	-20.25	4.00	10.43	3.70	2.16	1.81	-49.90
LC-5	0.25	0.137	0.549	0.824	3.68	2.91	-20.90	4.00	8.71	3.71	0.71	1.87	-49.11
LC-6	0.5	0.267	0.535	0.802	3.72	2.99	-19.74	4.01	7.64	3.73	0.18	2.06	-44.79
LC-7	0.75	0.384	0.512	0.768	3.83	3.09	-19.12	4.02	4.94	3.76	-1.69	2.29	-40.06
LC-8	1	0.485	0.485	0.728	3.87	3.21	-16.99	4.02	3.96	3.80	-1.87	2.54	-34.37
LC-9	1.333	0.595	0.446	0.669	3.96	3.37	-14.90	4.04	2.04	3.85	-2.75	2.84	-28.23
LC-10	2	0.743	0.371	0.557	4.02	3.61	-10.22	4.06	0.94	3.93	-2.28	3.28	-18.41
LC-11	4	0.912	0.228	0.342	4.11	3.93	-4.54	4.09	-0.61	4.04	-1.77	3.82	-7.22
LC-12	10	0.984	0.098	0.148	4.15	4.07	-1.78	4.10	-1.07	4.09	-1.28	4.05	-2.25
						MAX =	-1.78	MAX =	10.43	MAX =	2.16	MAX=	-2.25
						MIN =	-20.90	MIN =	-1.07	MIN =	-2.75	MIN =	-49.90
COMBINED L	OADS, C	C/B = 2.0								SQ-SUM			
	A/B	A	В	С	FEA	SRSS	% Dif	SQ-SUM	% Dif	ADJUSTED	% Dif	B31.3-EQ17	% Dif
LC-4	0.1	0.045	0.447	0.894	3.53	2.89	-18.22	3.87	9.63	3.63	2.83	1.58	-55.28
LC-5	0.25	0.111	0.444	0.889	3.58	2.90	-18.93	3.87	8.17	3.64	1.54	1.63	-54.60
LC-6	0.5	0.218	0.436	0.873	3.63	2.95	-18.63	3.88	6.91	3.65	0.63	1.77	-51.11
LC-7	0.75	0.318	0.424	0.848	3.69	3.03	-17.88	3.89	5.53	3.68	-0.27	1.98	-46.34
LC-8	1	0.408	0.408	0.816	3.76	3.12	-16.99	3.91	3.99	3.71	-1.25	2.21	-41.34
LC-9	1.333	0.512	0.384	0.768	3.80	3.25	-14.49	3.93	3.50	3.76	-1.05	2.50	-34.26
LC-10	2	0.667	0.333	0.667	3.95	3.48	-11.87	3.98	0.68	3.85	-2.56	2.98	-24.62
LC-11	4	0.873	0.218	0.436	4.09	3.85	-5.84	4.05	-0.94	4.00	-2.24	3.67	-10.37
LC-12	10	0.976	0.098	0.195	4.15	4.06	-2.19	4.10	-1.25	4.09	-1.51	4.02	-3.02
						MAX =	-2.19	MAX =	9.63	MAX =	2.83 MAX=		-3.02
						MIN =	-18.93	MIN =	-1.25	MIN =	-2.56 MIN =		-55.28
COMBINED L	OADS, C	C/B = 4.0								SQ-SUM			
	A/B	A	В	С	FEA	SRSS	% Dif	SQ-SUM	% Dif	ADJUSTED	% Dif	B31.3-EQ17	% Dif
LC-4	0.1	0.024	0.242	0.970	3.273	2.88	-11.86	3.50	6.87	3.36	2.64	1.20	-63.34
LC-5	0.25	0.061	0.242	0.968	3.289	2.89	-12.15	3.50	6.41	3.36	2.22	1.22	-62.92
LC-6	0.5	0.120	0.241	0.963	3.336	2.91	-12.91	3.51	5.12	3.37	1.05	1.29	-61.45
LC-7	0.75	0.179	0.239	0.954	3.37	2.93	-13.02	3.52	4.41	3.39	0.46	1.39	-58.83
LC-8	1	0.236	0.236	0.943	3.392	2.97	-12.58	3.53	4.19	3.40	0.38	1.51	-55.41
LC-9	1.333	0.308	0.231	0.923	3.443	3.02	-12.26	3.56	3.38	3.44	-0.18	1.70	-50.61
LC-10	2	0.436	0.218	0.873	3.5519	3.15	-11.21	3.62	1.96	3.51	-1.07	2.09	-41.14
LC-11	4	0.696	0.174	0.696	3.7987	3.53	-7.07	3.81	0.17	3.74	-1.53	2.99	-21.40
LC-12	10	0.925	0.092	0.370	4.087	3.95	-3.28	4.02	-1.54	4.01	-1.96	3.82	-6.43
						MAX =	-3.28	MAX =	6.87	MAX =	2.64	MAX=	-6.43
						MIN =	-13.02	MIN =	-1.54	MIN =	-1.96	MIN =	-63.34

 Notes:
 1. FEA results are the maximum stress intensity divided by the nominal bending stress in the pipe.

 2.
 This table was produced on an Excel spreadsheet. The number of significant figures is greater than indicated.

Table 2-5				
FEA Summary, Model 1	Combined Loads,	C/B = 10.0,	20.0, and	100.0

COMBINE	D LOADS	, C/B = 10	0.0							SQ-SUM			
	A/B	А	В	С	FEA	SRSS	% Dif	SQ-SUM	% Dif	ADJUSTED	% Dif	B31.3-EQ17	% Dif
LC-4	0.1	0.010	0.099	0.995	3.0576	2.88	-5.67	3.16	3.24	3.09	0.98	1.04	-66.11
LC-5	0.25	0.025	0.099	0.995	3.0592	2.88	-5.70	3.16	3.21	3.09	0.95	1.04	-65.99
LC-6	0.5	0.050	0.099	0.994	3.06	2.89	-5.63	3.16	3.25	3.09	0.99	1.05	-65.55
LC-7	0.75	0.074	0.099	0.992	3.073	2.89	-5.88	3.16	2.92	3.09	0.69	1.08	-64.95
LC-8	1	0.099	0.099	0.990	3.0929	2.90	-6.29	3.17	2.41	3.10	0.20	1.11	-64.18
LC-9	1.333	0.131	0.099	0.986	3.1169	2.91	-6.65	3.18	1.88	3.11	-0.29	1.16	-62.78
LC-10	2	0.195	0.098	0.976	3.1541	2.94	-6.79	3.20	1.39	3.13	-0.69	1.29	-58.98
LC-11	4	0.370	0.092	0.925	3.295	3.08	-6.52	3.30	0.24	3.25	-1.49	1.80	-45.44
LC-12	10	0.705	0.071	0.705	3.709	3.55	-4.41	3.66	-1.31	3.63	-2.11	2.99	-19.43
						MAX =	-4.41	MAX =	3.25	MAX =	0.99	MAX=	-19.43
						MIN =	-6.79	MIN =	-1.31	MIN =	-2.11	MIN =	-66.11
COMBINE	D LOADS	, C/B = 20	).0							SQ-SUM			
	A/B	А	В	С	FEA	SRSS	% Dif	SQ-SUM	% Dif	ADJUSTED	% Dif	B31.3-EQ17	% Dif
LC-4	0.1	0.005	0.050	0.999	2.974	2.88	-3.02	3.02	1.70	2.98	0.27	1.01	-66.06
LC-5	0.25	0.012	0.050	0.999	2.974	2.88	-3.02	3.02	1.70	2.98	0.27	1.01	-66.03
LC-6	0.5	0.025	0.050	0.998	2.977	2.88	-3.09	3.03	1.62	2.98	0.19	1.01	-65.94
LC-7	0.75	0.037	0.050	0.998	2.9778	2.89	-3.08	3.03	1.63	2.98	0.20	1.02	-65.75
LC-8	1	0.050	0.050	0.998	2.9781	2.89	-3.04	3.03	1.66	2.99	0.24	1.03	-65.47
LC-9	1.333	0.066	0.050	0.997	2.9778	2.89	-2.93	3.03	1.75	2.99	0.34	1.04	-64.97
LC-10	2	0.099	0.050	0.994	3.004	2.90	-3.51	3.04	1.10	3.00	-0.30	1.08	-63.92
LC-11	4	0.196	0.049	0.979	3.067	2.94	-4.13	3.07	0.20	3.03	-1.12	1.28	-58.42
LC-12	10	0.447	0.045	0.894	3.278	3.17	-3.41	3.27	-0.27	3.24	-1.22	2.04	-37.62
						MAX =	-2.93	MAX =	1.75	MAX =	0.34	MAX=	-37.62
						MIN =	-4.13	MIN =	-0.27	MIN =	-1.22	MIN =	-66.06
COMBINE	D LOADS	, C/B = 10	0.0							SQ-SUM			
	A/B	А	В	С	FEA	SRSS	% Dif	SQ-SUM	% Dif	ADJUSTED	% Dif	B31.3-EQ17	% Dif
LC-4	4	0.040	0.010	0.999	2.907	2.89	-0.71	2.91	0.27	2.90	-0.23	1.01	-65.15
LC-5	10	0.099	0.010	0.995	2.928	2.90	-1.00	2.93	-0.04	2.91	-0.53	1.08	-63.25
LC-6	30	0.287	0.010	0.958	3.052	3.00	-1.58	3.03	-0.75	3.02	-1.17	1.52	-50.19
LC-7	60	0.514	0.009	0.857	3.328	3.25	-2.26	3.27	-1.70	3.26	-1.99	2.28	-31.48
LC-8	100	0.707	0.007	0.707	3.6383	3.55	-2.47	3.56	-2.15	3.55	-2.31	2.99	-17.85
LC-9	200	0.894	0.004	0.447	3.9733	3.89	-2.01	3.90	-1.91	3.90	-1.96	3.70	-6.87
LC-10	500	0.981	0.002	0.196	4.0946	4.07	-0.68	4.07	-0.66	4.07	-0.67	4.03	-1.53
LC-11	1000	0.995	0.001	0.100	4.1012	4.10	-0.11	4.10	-0.11	4.10	-0.11	4.09	-0.33
LC-12	10000	1.000	0.000	0.010	4.1086	4.11	-0.04	4.11	-0.04	4.11	-0.04	4.11	-0.04
						MAX =	-0.04	MAX =	0.27	MAX =	-0.04	MAX=	-0.04
						MIN =	-2.47	MIN =	-2.15	MIN =	-2.31	MIN =	-65.15

 Notes:
 1. FEA results are the maximum stress intensity divided by the nominal bending stress in the pipe.

 2.
 This table was produced on an Excel spreadsheet. The number of significant figures is greater than indicated.

#### Table 2-6 FEA Results

Mip =A'	M	Mop=B*I	M	Mt = C*I	M									
					Model									
C/B	A/B	А	В	С	1	2	3	4	5	6	7	8	9	10
0	0.100	0.100	0.995	0.000	2.91	8.13	3.55	2.05	10.81	4.43	5.57	4.85	5.94	3.26
	0.250	0.243	0.970	0.000	2.99	8.69	3.74	2.07	11.29	4.62	5.92	4.97	6.20	3.30
	0.500	0.447	0.894	0.000	3.20	10.14	4.07	2.13	13.18	5.10	6.63	5.35	6.78	3.44
	0.750	0.600	0.800	0.000	3.44	11.11	4.34	2.20	14.27	5.57	7.14	5.66	7.38	3.61
	1.000	0.707	0.707	0.000	3.63	11.63	4.63	2.25	14.81	5.93	7.63	5.93	7.78	3.79
	1.333	0.800	0.600	0.000	3.79	12.45	4.83	2.33	15.89	6.29	8.08	6.20	8.04	3.99
	2.00	0.894	0.447	0.000	3.97	13.26	5.03	2.42	17.03	6.58	8.43	6.38	8.41	4.19
	4.00	0.970	0.243	0.000	4.09	13.72	5.20	2.49	17.67	6.82	8.68	6.65	8.78	4.38
	10.00	0.995	0.100	0.000	4.10	13.70	5.18	2.52	17.70	6.92	8.83	6.73	8.84	4.42
0.1	0.100	0.099	0.990	0.099	3.09	8.78	3.81	2.13	11.63	4.74	6.00	5.03	6.25	3.40
	0.250	0.241	0.966	0.097	3.17	9.33	3.98	2.15	12.03	4.93	6.34	5.17	6.40	3.45
	0.500	0.445	0.891	0.089	3.36	10.51	4.26	2.20	13.72	5.32	6.93	5.47	6.98	3.56
	0.750	0.598	0.797	0.080	3.56	11.44	4.52	2.26	14.76	5.77	7.42	5.78	7.47	3.72
	1.000	0.705	0.705	0.071	3.71	11.94	4.74	2.30	15.27	6.02	7.76	5.98	7.87	3.89
	1.333	0.798	0.599	0.060	3.86	12.49	4.92	2.37	16.00	6.38	8.20	6.25	8.13	4.04
	2.00	0.894	0.447	0.045	4.00	13.31	5.07	2.44	17.11	6.65	8.52	6.43	8.41	4.23
	4.00	0.970	0.242	0.024	4.11	13.76	5.22	2.49	17.74	6.83	8.69	6.66	8.80	4.38
	10.00	0.995	0.099	0.010	4.10	13.72	5.19	2.52	17.73	6.93	8.84	6.74	8.85	4.43
0.25	0.100	0.097	0.966	0.241	3.31	9.59	4.14	2.23	12.69	5.12	6.53	5.23	6.61	3.57
	0.250	0.236	0.943	0.236	3.39	10.11	4.28	2.25	13.06	5.30	6.85	5.41	6.74	3.63
	0.500	0.436	0.873	0.218	3.55	10.91	4.49	2.29	14.34	5.61	7.29	5.59	7.18	3.75
	0.750	0.588	0.784	0.196	3.71	11.83	4.73	2.33	15.36	6.00	7.75	5.91	7.53	3.87
	1.000	0.696	0.696	0.174	3.80	12.31	4.86	2.37	15.84	6.21	7.94	6.04	7.95	4.01
	1.333	0.791	0.593	0.148	3.94	12.54	5.03	2.42	16.06	6.46	8.32	6.30	8.22	4.10
	2.00	0.889	0.444	0.111	4.02	13.33	5.11	2.46	17.18	6.73	8.63	6.49	8.42	4.28
	4.00	0.968	0.242	0.061	4.13	13.80	5.25	2.50	17.81	6.83	8.69	6.66	8.81	4.39
	10.00	0.995	0.099	0.025	4.10	13.75	5.20	2.52	17.76	6.93	8.84	6.74	8.86	4.43
0.5	0.100	0.089	0.891	0.445	3.56	10.46	4.49	2.33	13.81	5.55	7.11	5.43	6.96	3.74
	0.250	0.218	0.873	0.436	3.63	10.94	4.60	2.35	14.15	5.69	7.41	5.61	7.07	3.82
	0.500	0.408	0.816	0.408	3.76	11.31	4.76	2.38	14.90	5.98	7.62	5.74	7.42	3.91
	0.750	0.557	0.743	0.371	3.86	12.15	4.94	2.41	15.90	6.22	8.08	5.98	7.63	4.03
	1.000	0.667	0.667	0.333	3.95	12.64	5.03	2.44	16.38	6.43	8.28	6.15	7.93	4.12
	1.333	0.766	0.575	0.287	4.01	12.89	5.13	2.46	16.55	6.52	8.40	6.30	8.26	4.21
	2.00	0.873	0.436	0.218	4.09	13.27	5.22	2.49	17.14	6.74	8.74	6.54	8.45	4.32
	4.00	0.963	0.241	0.120	4.10	13.83	5.28	2.52	17.88	6.82	8.74	6.65	8.80	4.39
	10.00	0.994	0.099	0.050	4.10	13.79	5.22	2.52	17.82	6.93	8.84	6.75	8.87	4.44
1	0.100	0.071	0.705	0.705	3.69	10.89	4.69	2.38	14.41	5.77	7.39	5.50	7.09	3.82
	0.250	0.174	0.696	0.696	3.75	11.30	4.74	2.38	14.72	5.84	7.65	5.65	7.20	3.88
	0.500	0.333	0.667	0.667	3.84	11.70	4.91	2.42	14.87	6.13	7.89	5.77	7.47	3.97
	0.750	0.469	0.625	0.625	3.91	11.91	4.97	2.45	15.77	6.25	8.07	5.96	7.74	4.08
	1.000	0.577	0.577	0.577	3.98	12.52	5.10	2.47	16.41	6.43	8.35	6.08	7.84	4.13
	1.333	0.686	0.515	0.515	4.04	12.95	5.16	2.49	16.80	6.60	8.50	6.24	8.15	4.22
	2.00	0.816	0.408	0.408	4.12	13.12	5.27	2.51	16.79	6.73	8.68	6.47	8.43	4.28
	4.00	0.943	0.236	0.236	4.16	13.78	5.28	2.53	17.84	6.89	8.85	6.62	8.75	4.39
	10.00	0.990	0.099	0.099	4.13	13.84	5.29	2.52	17.89	6.93	8.83	6.74	8.88	4.44

Notes:

1. FEA results are the maximum stress intensity divided by the nominal bending stress in the pipe.

2. This table was produced on an Excel spreadsheet. The number of significant figures is greater than indicated.

## 2.3 Analysis of Results

Tables 2-2 and 2-3 list the results of the FEA for model 1. Load cases LC-1, LC-2, and LC-3 represent the response to in-plane, out-of-plane and torsion moments. As stated earlier, these results are defined as S1, S2, and S3 respectively. For each of the six values of C/B, there are eight load cases (LC-4 to LC-12). The values of A, B, and C are also listed. The column listed as "FEA" is the maximum stress intensity corresponding to the loads of the particular load case. Note that in all cases, LC-2 is equal to LC-3. This is due to the symmetry of 90° elbows discussed earlier.

To check results, the values of S1, S2, and S3 for all of the models were compared to the "secondary" stress indices as defined in Code Case N-319-2 [8]. The differences were very small (see Table 2-1); the average difference for in-plane bending was 1.8% and 8.6% for out-of-plane or torsion. The maximum differences were 6.7% and 15.4%. This served to verify the results.

By using eq. 2-4, the magnitude of the applied moment was kept constant. Load cases 1 to 3 are defined in COSMOS as "primary load cases." The other load cases are defined as "secondary" and are obtained by scaling and superposing the results of the primary cases. (These definitions should not be confused with primary and secondary stresses, defined in ASME Section III). This uses the principal of superposition and thus is valid only for linear analysis. For in-plane bending (LC-1), the stress distribution is symmetric about the x-y plane (Figure 1-1). However, the stress distribution for out-of-plane bending and torsion are not symmetrical about the x-y plane. Thus, the results of a moment vector defined by the parameters A, B, and C may not have the same local stress results as the moment defined by A, -B, and C, even though the magnitude of the moment is equal. For example,

$$(A2+B2+C2)1/2 = (A2+(-B)2+C2)1/2$$
 (equation 2-5)

In investigating this, the maximum stress intensity for various load combinations where the signs of the moment components were varied were determined for model 1 listed in Table 2-2. From this study, it is concluded that the following sets of load multipliers will yield the same results:

Set 1: (+A, +B, +C), (+A,-B, -C), (-A, +B, +C), (-A, -B, -C)

Set 2: (+A, -B, +C), (+A, +B, -C), (-A, -B,+C), (-A,+B, -C)

Within each set, the maximum stress intensities were essentially identical. The stress intensity obtained from Set 2 is less than that derived from Set 1.

In order to be conservative, the combination method will always follow that of Set 1.

## 2.4 Combination Methodologies

In order to evaluate existing and potential new methods of combining the effects of different moments, various combination methods were investigated. The methods considered are discussed in the following sections. Tables 2-2 through 2-5 contain detailed comparisons of the

FEA and the various combination methodologies discussed in the following sections for model 1. Table 2-7 contains a summary for other models investigated.

## 2.5 SRSS Combination Methodology

The values listed in the SRSS column in Tables 2-2 through 2-5 and 2-7 uses the "square root sum of the squares methodology." This approach uses the FEA results for the pure in-plane, out-of-plane, and torsion as a basis for evaluating the effects of various combinations. SRSS is calculated using:

SRSS =  $((A*S1)^2 + (B*S2)^2 + (C*S3)^2)^{1/2}$  (equation 2-6)

A review of the tables indicates that for the models investigated, for the case with no torsion (C=0), the maximum percentage difference between the FEA and SRSS results occurs in model 5 and is 7.8%. The average percentage difference for all the models is small, only a few percent.

However, when torsion is included (for example, C/B is greater than 0), the percentage difference increases to a maximum of -29.3% (Test 2) and an average of about 7.69%. The standard deviation is 7.05%, and the correlation factors squared is  $r^2 = 0.973$ . (See Table 2-7.)

	Combination Methodology	SRSS	SQ-SUM	SQ-SUM (ADJUSTED)	B31.3- EQ17
Model 1	Maximum	21.9	2.23	3.92	41.1
	Minimum	0.108	-10.6	-1.96	0.108
	Average	7.46	-1.05	1.38	10.3
Model 2	Maximum	29.3	9.17	11.5	48.6
	Minimum	1.40	-0.748	1.40	1.40
	Average	11.1	4.04	6.07	13.8
Model 3	Maximum	25.6	4.85	8.05	45.0
	Minimum	0.573	-4.99	0.573	0.57
	Average	9.81	1.80	4.09	12.6
Model 4	Maximum	14.7	0.669	0.669	32.6
	Minimum	-1.29	-20.4	-9.41	-1.29
	Average	4.58	-5.17	-2.39	7.39
Model 5	Maximum	27.5	7.8	9.76	48.1
	Minimum	1.43	-2.17	1.43	1.43
	Average	10.1	2.78	4.89	12.9
Model 6	Maximum	24.8	2.98	7.20	45.1
	Minimum	0.426	-6.00	0.0780	0.426
	Average	8.28	0.486	2.72	11.1
Model 7	Maximum	28.3	6.42	10.4	47.4
	Minimum	0.190	-2.37	-0.002	0.19
	Average	9.88	2.48	4.60	12.6
Model 8	Maximum	13.8	2.06	2.06	36.5
	Minimum	0.0648	-23.5	-11.4	0.0648
	Average	4.56	-4.66	-2.02	7.99
Model 9	Maximum	17.0	4.31	4.31	40.1
	Minimum	0.715	-17.1	-6.28	0.715
	Average	6.17	-2.44	0.0283	9.40
Model 10	Maximum	16.3	1.04	1.04	37.3
	Minimum	-2.00	-18.9	-8.19	-2.00
	Average	4.87	-4.25	-1.64	8.04
All	Maximum	29.3	9.17	11.5	48.6
Models	Minimum	-2.00	-23.5	-11.4	-2.00
	Average	7.69	-0.598	1.77	10.6
	STDEV=	7.05	5.21	3.87	11.4
	$r^2 =$	0.973	0.995	0.996	0.932

 Table 2-7

 Summary of FEA and Expressions—Percentage Difference from FEA Results

Notes:

- 1. The Maximum, Minimum, Average, and STDEV are calculated from the % differences between the FEA results and the specific combination methodology. The  $r^2$  is based on a comparison between the FEA and combination methodology calculation.
- 2. This table was produced on an Excel spreadsheet. The number of significant figures is greater than indicated.

#### 2.6 B31.3-EQ17 Combination Methodology

As discussed earlier, in B31.3 the expression (equation 17 of B31.3) used to evaluate thermal expansion stresses is:

$$S_{E} = [S_{b}^{2} + 4S_{t}^{2}]^{1/2}$$
 (equation 2-7)

where  $S_{h}$  is the resultant bending stress given by:

$$S_{b} = [(i_{i}M_{i})^{2} + (i_{o}M_{o})^{2}]/Z$$
 (equation 2-8)

and  $S_{t}$  is the torsional stress given by  $M_{t}/2Z$ .

Here, the stresses due to torsion are not intensified. In order to evaluate this approach to combining the effects of the different moments, the equivalent representation would be (using B31.1-EQ17 to represent this approach):

B31.1-EQ17 = 
$$((A*S1)^2 + (B*S2)^2 + (C)^2)^{1/2}$$
 (equation 2-9)

The assumption for eq. 2-9 is that A\*S1 and B\*S2 correspond to an "intensified" stress due to inplane and out-of-plane moments, where A and B are related to the magnitude of the moment. For torsion, instead of using S3 in the calculations, it is replaced by the value 1.0 (for example, not intensified).

In Tables 2-2, 2-3 and 2-7, the B31.1-EQ17 column lists the values calculated using this approach. The maximum percentage difference is 48.7, the average is 10.6%, the standard deviation is 11.5%, and  $r^2 = .932$ . The maximum difference was for model 2, C/B = 1.0, LC-4.

## 2.7 SQ-SUM Combination Methodology

 $a+b(C/B)^n$ 

In this formulation, the effects from the out-of-plane and torsion were added directly. The result would then be "square root sum of the squares" with the in-plane bending effects. The representation is:

$$SQ-SUM = ((A*S1)^2 + (B*S2 + C*S3)^2)^{1/2}$$
 (equation 2-10)

These results are listed in Tables 2-2 through 2-5 and 2-7. The maximum percentage difference is 23.5% for model 8 which has R = 18 in. The average of the maximum differences for all of the 10 models is -0.58%, the standard deviation is 5.2%, and  $r^2 = .995$ .

## 2.8 SQ-SUM (ADJUSTED) Combination Methodology

This approach is very similar to the SQ-SUM approach except the following adjustment factor is added:

(equation 2-11)

2-13

where a, b and n are constants and C/B is the ratio of the magnitude of the torsion to the out-ofplane bending moment at the end of the elbow. The expression is

SQ-SUM (ADJUSTED) = 
$$((A*S1)^2 + (a+b(C/B)^n)*(B*S2+C*S3)^2)^{1/2}$$
 (equation 2-12)

If C/B > 1, replace C/B with B/C; see Section 2.9.

The results are also listed in Tables 2-2, 2-3 and 2-7. For the values of the constants given by:

a = 1.0 (equation 2-13) b = -0.187n = 0.635

the maximum percentage difference is 11.5%, the average of the maximum differences for each of the 10 models is 1.8%, the standard deviation is 0.387, and  $r^2$  is .996.

## 2.9 Conditions When C/B $\geq$ 1.0 for SQ-SUM (ADJUSTED) Methodology

The FEAs discussed earlier were for the condition where the applied out-of-plane bending moment was greater than or equal to the torsional moment, for example,  $C/B \le 1.0$ . Because of the symmetry of the 90° elbow and considering that the torsion on one end results in equal out-of-plane bending on the other (and vice versa) if  $C/B \ge 1.0$  at the end being evaluated, the same expression can be used to predict the stress intensity by replacing C/B with B/C. In order to confirm this, additional runs were made for model 1. The results are listed in Tables 2-4 and 2-5. The additional cases were for C/B equal to 1.5, 2.0, 4.0, 10.0, 20.0 and 100.0. For C/B equal to 100, the values of A/B were taken to be 4, 10, 30, 60, 100, 200, 500, 1000, and 10,000. The maximum percentage difference for the SQ-SUM (ADJUSTED) combination methodology was 2.84%, which confirms the methodology of using B/C for C/B when C/B  $\ge 1.0$ .

## 2.10 Elbow Flexibility

Piping system design is based on an analytical determination of displacements, rotations, moments, and reaction forces at various positions along a piping system. The analysis is based on a description of the piping system as an interconnected set of straight and curved beams. Flexibility factors are introduced into the analytical model to correct for the differences in structural behavior between the beam model and the piping system components that make up the real piping system. Adequate characterization of the flexibility of piping components is essential to correctly estimating pipe stresses and support loads. Properly modeling the flexibility of an elbow is at least as significant as the accurate determination of stress intensification factors for the elbows.
The flexibility of an elbow as defined by the Code [1] is 1.65/h, where h is equal to  $tR/r^2$ . As stated previously, when using Code Case N-319-2, the flexibility factors provided in the Code Case must be used in pipe stress evaluations. The Code Case uses:

$k_{y} = 1.25/h$ when the internal pressure is equal to zero.	(equation 2-14)

 $k_z = 1.3/h$  when internal pressure is equal to zero. (equation 2-15)

Tables 2-8 and 2-9 show the results of FEA analysis of elbow flexibility for four cases with h ranging from .66 to 0.06. The basic model was for  $D_0 = 10$  in. (25.4 cm). The equations used in the tables to calculate the flexibility factors from the FEA deflection results were obtained from [5], which gives the background for Code Case N-319-2. The results as calculated from the Code Case equations are also included. These values are close. As can be seen from the tables, the magnitude of the elbow flexibility factors can be significant, especially for thin wall systems.

## Table 2-8 In-Plane Flexibility Calculations

Case	t	Ux	Uy	Rz	k <sub>xz</sub>	k <sub>yz</sub>	k <sub>zz</sub>	h	1.3/h
1	0.909	-1.09E-01	-3.52E-01	8.98E-03	2.31	2.15	2.17	0.660	1.97
2	0.196	-2.01E-01	-1.15E+00	2.49E-02	12.48	12.20	12.2	0.122	10.6
3	0.091	-3.11E-01	-2.15E+00	4.50E-02	25.33	25.01	25.0	0.061	21.5
4	0.530	-1.28E-01	-5.05E-01	1.20E-02	4.20	4.01	4.04	0.355	3.67

Notes:

1. My = In-plane moment

2.  $k_x = [-\delta_x/(M/EI) - 0.5L^2 - 0.707RL]/(0.571R^2)$ 

3.  $k_y = [-\delta_y/(M/EI) - 1.5L^2 - RL]/[R(1.571L + R)]$ 

4.  $k_z = [\theta_z/(M/EI) - 2L]/(1.571R)$ 

5.  $E = 3*10^7$  psi

- 6.  $I = 0.0491 * (D_0^4 (D_0 t_n)^4)$
- 7.  $R = 15 \text{ in.}, D_{o} = 10 \text{ in.}$

8.  $L = 4*(D_o - t_n)$ 

## Table 2-9 Out-of-Plane Flexibility Calculations

Case	Uz	Rx	Ry	<b>k</b> <sub>xy</sub>	k <sub>yy</sub>	k <sub>zy</sub>	h	1.25/h
1	3.56E-01	5.40E-04	8.92E-03	2.29	2.03	2.04	0.66	1.89
2	7.53E-01	6.39E-03	1.62E-02	13.8	11.3	11.8	0.12	10.2
3	1.24E+00	1.37E-02	2.55E-02	28.4	23.1	24.3	0.06	20.6
4	4.35E-01	1.67E-03	1.03E-02	4.48	3.73	3.86	0.35	3.53

Notes:

1. My = out-of-plane moment

2.  $k_x = [\theta_x/(M/EI) + 0.65R]/(0.5R)$ 

3.  $k_y = [\theta_y/(M/EI) - 2.3L - 1.021R]/(0.785R)$ 

4.  $k_z = [\delta_z/(M/EI) - 1.8L^2 - 2.321RL - 0.65R^2] / [R(0.7854L + 0.5R)]$ 

5.  $E = 3*10^7$  psi

6.  $I = 0.0491 * (D_o^4 - (D_o - t_n)^4)$ 

7. R = 15 in.,  $D_0 = 10$  in.

8.  $L = 4*(D_o - t_n)$ 

Finite Element Analysis Investigation

# 2.11 Elbow Characteristic "h" Effects

As indicated in Table 2-1, the FEA models used in the evaluation of the methods of combing the effects of different loading conditions had a range of elbow characteristics from 0.036 to 0.626. In order to investigate the effects of h, additional FEA was performed on additional models.

Three models with h values of 0.500, 0.637, and 0.809 were studied. The results are summarized in Table 2-10. As before, the percentage differences from the FEA to the different methods are listed. The results are similar to those listed in Table 2-4.

						Expression			
Model	Do	Т	R	h		SRSS	SQ-Sum	SQ- SUM ADJUSTED	B31.3- EQ17
11	4.5	0.531	6	0.809	Maximum	0.53	30.43	17.64	-1.52
					Minimum	-10.2	-9.68	-9.68	-27.5
					Average	-6.93	3.69	0.66	-9.9
12	4.5	0.5	4	0.500	Maximum	-2.77	20.94	9.09	-2.77
(TOP)					Minimum	-17.0	-13.1	-13.1	-34.8
					Average	-11.2	-4.05	-6.21	-14.0
13	4.5	0.438	6	0.637	Maximum	-0.40	27.7	15.2	33.0
(TOP)					Minimum	-10.1	-6.61	-6.61	-28.9
					Average	-4.72	5.7	2.72	-7.81
13	4.5	0.438	6	0.637	Maximum	3.71	16.4	5.32	3.48
(BOTTOM)					Minimum	-17.6	-5.48	-5.48	-33.9
					Average	-6.58	2.66	0.016	-9.11

Table 2-10 Characteristic "h" Effects—Percentage Difference from FEA Results

It was noted that the maximum stresses were located differently than for the models with lower values of h. In general for thin wall elbows, the maximum stress is on the inside of the elbow. However, for thicker elbows, it can be on the outside. The results in Table 2-4 are the "worst case" results from the inside and outside surfaces.

In addition, model 13 was analyzed using thick shell elements to determine if this had any effects. The maximum difference in S1, S2, or S3 was less than 2%.

From this evaluation, it is deemed that the methodology suggested in Section 2.8 is valid for values of  $h \le 0.8$ .

# **3** TEST PROGRAM

## 3.1 Purpose

The purpose of this test program was to obtain some specific data which would provide insight into the effects of the direction of the loading. As discussed earlier, the SIFs presently used are based on both in-plane and out-of-plane tests performed by Markl [6]. However, there are no published results on tests where the loading is a combination of these conditions. Test data for combined loading would serve as a basis for establishing a methodology for evaluating directionality effects.

# 3.2 Design of Test Specimens

The test specimens consisted of 4 in. NPS schedule 40, A106, GrB long radius elbows. Eight specimens were fabricated by Energy Northwest. Two specimens were fabricated and tested for in-plane bending, two for out-of-plane bending, and four for bending at a 45° angle. While the focus of this investigation is for loadings that are not strictly in-plane or out-of-plane, the tests for in-plane and out-of-plane loads would provide a benchmark for the tests in general.

The effect of the configuration at a  $45^{\circ}$  is that the loading is equivalent to a combined in-plane and out-of-plane moment loading where the moments are equal. This loading condition is designated "combined  $45^{\circ}$ " loading.

Figure 3-1 has schematics of the three test configurations. Figure 3-2 shows photographs of the actual test setups.

## Test Program





Test Program



(a) In-Plane Bending



(b) Out-of-Plane Bending



(c) Combined (45') Bending

Figure 3-2 Test Setups Test Program

# 3.3 Testing Program

Testing was performed at Ohio State University. The fatigue tests were performed on an MTS Systems Corporation Series 319 dynamically rated Axial/Torsional Load Frame. This unit is designed to accommodate either uniaxial or multiaxial testing. Load Frame capacities are 55,000 lb axial force and 20,000 in-lb torsional moment. A computerized control panel provides local, precise operations of the cross head, hydraulic grips, and actuator. The maximum actuator displacement is six inches. The loading pattern applied to an attached sample is controlled by programmable servo valves.

Built-in loading programs include sinusoidal and triangular waves with the user being able to select, within machine limits, the desired amplitude and frequency. The actual displacement of the actuator is measured by a linear variable differential transformer (LVDT). The output of either the load cell or the LVDT can be selected for closed loop control of the actuator displacement time history. During a test, the number of cycles of applied load is recorded by a digital counter and displayed on the MTS console.

In these tests, the load was sinusoidal at frequencies ranging from 0.3 to 0.5 Hz. Actuator displacement was designated as the test control variable. The selection of displacement as the control parameter meant that actuator movement was used by the MTS system for feedback in the closed loop controls. This resulted in virtually identical cycles of actuator displacement being recorded throughout the duration of each test. The load resulting from the imposition of the specified displacement was measured with fatigue-rated, 5000-lb capacity, tension-comparison, electric load cell manufactured by the Lebow Instrument Company. The output of this load cell was monitored continuously throughout the duration of each test.

Both load and actuator displacements were recorded using a computer program written at OSU, in LabVIEW, specifically for that purpose. LabVIEW is a graphical language developed by National Instruments that allows the user to design in software a test control and data collection system tailored to the requirements of each experimental program. In the LabVIEW application developed for the fatigue tests, the signals from the load and displacement transducers were sampled 30 times per second, and the time histories of each were plotted on the computer screen in real time so that the progress of the test could be readily monitored. By combining the load and displacement time histories, a plot of load vs. displacement at any load cycle desired could be constructed. This too was done in real time so that the changes in the response of the test specimens could be identified while the specimen was still undergoing loading. Any of these presentations of the test data could be printed while the test was still in progress.

Figure 3-1 shows the load application point and direction of loading. Note that the measured distance from the load point to the centerline of the pipe ( $\sim$ 49.25 in.) did not vary for the test specimens. The measured distance (L), which is dependent on the installation, is included in the test data.

The test data, results, and other information are provided in Appendix A. The tests were displacement-controlled cantilever bending tests. The tests followed the standard approach corresponding to Markl type tests [6]. Each specimen was first tested to determine the load deflection curve for that particular specimen. The load deflection curve was used to determine

the stiffness of each specimen and the load applied to the specimen by a given amount of displacement. The load deflection curves were determined for loading in both positive and negative loading directions (down and up). Each specimen was then fatigue tested by cycling the deflection in both directions of loading by a controlled amount. The cycles to failure were counted to determine the fatigue life. Failure was detected when wall cracks formed and water leaked though the cracks.

The results of Table 3-1 are based on nominal dimensions, (for example, D = 4.5 in. and t = 0.237 in.). In order to determine the actual dimensions, the test specimens were sectioned so that measurements could be taken. The average measurements were OD = 4.58 in. and t = 0.276 in. The distance from the load point to the point of failure was also measured.

Table 3-1	
Summary of Te	st Results

Test	Load Type	F	М	N <sub>eq</sub>	i	Average i
		lbs.	in. Ibs.	Cycles to Failure	Note(2)	for each Ioad type
Α	In-plane	2094	103130	2080	1.658	
В	In-plane	1940	95545	2890	1.675	1.667
С	Out-of-plane	2490	122633	2490	1.253	
D	Out-of-plane	2537	124947	2879	1.282	1.268
Е	Combined (45°)	2088	102782	3344	1.512	
F	Combined (45°)	2105	103671	2917	1.541	
G	Combined (45°)	2135	105149	2157	1.614	
Н	Combined (45°)	2194	108055	2440	1.533	1.550

Notes:

1. M = F L, where L = 49.25"

2. The value of i is calculated from i = 245,000  $N_{eq}^{-0.2}/S$ , where Neq = equivalent cycles to failure, and

S = M/Z. Where there were more than one loading conditions, Neq was calculated using: Neq =  $S (\delta_i / \delta_{max})^{5*} N_i$  where  $\delta_{max}$  is the maximum displacement,  $\delta_i$  is the "i" th displacement and Ni is the number of cycles associated with the "i"th displacement.

- 3. Z = 3.215 in.<sup>3</sup> Based on nominal dimensions for the elbow.
- 4. Tests A, B, C and H used two loading conditions. All others used one loading condition.
- 5. This table was produced on an Excel spreadsheet. The number of significant figures is greater than indicated.

# **4** ANALYSIS OF TEST RESULTS

# 4.1 Introduction

In this section, the test results will be compared to the FEA calculations and Code expressions.

## 4.2 Comparison to FEA

Table 4-1 lists the test results and also the FEA results for model 1 for the same loading. The FEA results are based on nominal dimensions corresponding to the test specimens. The ratio of test SIF to FEA results is also provided. The average ratio of test SIF/FEA is .425.

TEST	LOAD TYPE	i	FEA	i/FEA
Α	In-plane	1.66	4.11	0.403
В	In-plane	1.68	4.11	0.408
С	Out-of-plane	1.25	2.88	0.435
D	Out-of-plane	1.28	2.88	0.445
E	Combined (45°)	1.51	3.63	0.417
F	Combined (45°)	1.54	3.63	0.425
G	Combined (45°)	1.61	3.63	0.445
н	Combined (45°)	1.53	3.63	0.422
			Average =	0.425

# Table 4-1 Summary of Test Results Based on Nominal Dimensions

Notes:

- 1. All calculations are based on nominal dimensions.
- 2. This table was produced on an Excel spreadsheet. The number of significant figures is greater

than indicated.

3. FEA results are the maximum stress intensity divided by the nominal bending stress in the pipe.

From eq. 1-10:

$$i = C_2 K_2 / 2$$

(equation 1-10)

Analysis of Test Results

it is expected that since  $C_2K_2$  corresponds to the FEA results, the ratio of SIF/FEA would be about 0.5.

The evaluation of the FEA for model 1 was based on nominal dimensions ( $D_o = 4.5$  in.,  $t_n = 0.237$  in.). As noted in Section 3, the actual dimensions were:  $D_o = 4.58$  in. and  $t_n = 0.276$  in. For comparison, the FEA was rerun with actual dimensions for the case with no torsion (C/B = 0). The test SIFs were also recalculated using the measured dimensions and measured distance L to the point of failure. Table 4-2 lists the results and includes the ratio of the SIFs from the test data to the FEA results. As seen from Table 4-2, the average ratio is 0.56 as compared to 0.50 as expected from eq. 1-10.

i(test)/i

1.13 1.15 1.04 1.07 1.10 1.12 1.17

1.11

1.11

50	ounmary of rest results based on Actual Dimensions									
Test	Load Type	i(test)	FEA	i/FEA	C <sub>2</sub> (N-319-2)	i(test)/C <sub>2</sub>	i = 0.9/h <sup>2/3</sup> or i = 0.75/h <sup>2/3</sup>			
					(Note 3)		(Note 4)			
Α	In-plane	2.03	3.73	0.543	3.87	0.524	1.79			
В	In-plane	2.05	3.73	0.549	3.87	0.529	1.79			
С	Out-of-plane	1.55	2.68	0.580	3.04	0.510	1.49			
D	Out-of-plane	1.60	2.68	0.598	3.04	0.526	1.49			
Е	Combined (45°)	1.80	3.31	0.544	3.48	0.518	1.64			
F	Combined (45°)	1.84	3.31	0.554	3.48	0.527	1.64			
G	Combined (45°)	1.92	3.31	0.580	3.48	0.553	1.64			

l able 4-2	
Summary o	Test Results Based on Actual Dimensions

Notes:

н

1. All calculations are based on actual dimensions; Do = 4.58",  $t_n = 0.276$ ", R = 6". This results in h = 0.3576.

3.31

Average =

1.83

2. L is to point of failure.

Combined (45°)

3. The values for  $C_2$  are based on Code Case N-319-2. See eq. 1-11 and eq. 1-12. For the combined loading, the approach suggested by N-319-2 is followed. (See eq. 1-13.) Since the in-plane and out-of-plane moments are equal, this is equivalent to  $0.7071(C_{2Z}^{2}+C_{2X}^{-2})^{1/2}$ .

0.551

0.562

3.48

Average =

0.525

0.526

1.64

Average =

4. The values of i for in-plane and out-of-plane loading are based on eq. 1-8 and eq. 1-9: i (inplane) =  $0.9/h^{2/3}$  and i (out-of-plane) =  $0.75/h^{2/3}$ . For the combined loading,  $0.7071((i_i^2+i_o^2)^{1/2})$  is used.

5. This table was produced on an Excel spreadsheet. The number of significant figures is greater than indicated.

6. FEA results are the maximum stress intensity divided by the nominal bending stress in the pipe.

# 4.3 Comparison to Code Requirements

Table 4-2 also provides a comparison of the test SIFs to  $C_2$  as calculated using Code Case N-319-2. For the combined loading, the "equivalent"  $C_2$  is calculated using the square root sum of the squares approach as suggested by Code Case N-319-2. This is discussed in the notes to

Analysis of Test Results

Table 4-2. The average value of the ratio of the test SIFs to  $C_2$  is 0.53 versus 0.50 as suggested by eq. 1-10.

Table 4-2 also includes a comparison to the SIFs as defined in the various Codes. As discussed earlier, ASME Section III provides only one SIF for all loading conditions. B31.3 uses the same expression for in-plane loading and a different one for out-of-plane bending. These expressions are repeated below:

$i = 0.9/h^{2/3}$ for in-plane bending	(equation 1-1)

 $i = 0.75/h^{2/3}$  for out-of-plane bending (equation 1-5)

Table 4-2 includes a comparison of equations 1-1 and 1-5 to the test results. The results are very close.

The test loading at  $45^{\circ}$  was compared to the SRSS of the in-plane and out-of-plane SIFs. This was based on the B31.3 approach to evaluating combined loading conditions. For combined loading, the following expression is used:

$$S_{b} = [(i_{i}M_{i})^{2} + (i_{o}M_{o})^{2}]^{0.5}/Z$$

(equation 4-1)

This is discussed in the notes to Table 4-2.

The average ratio of the test SIFs to the calculated SIFs is 1.11.

# 4.4 Summary

In summary, considering the typical variability in fatigue-related test data and other uncertainties, the results are remarkably accurate and consistent.

# 5 CONCLUSIONS

The following conclusions, particularly with respect to "conservative" and "unconservative," are based on the assumption that the moments derived from piping system analyses are accurate. The conclusions arrived at from the analyses and tests discussed in this report are enumerated below

- 1. Code Case N-319-2 uses the expression  $[(C_{2x}M_x)^2+(C_{2y}M_y)^2+(C_{2z}M_z)^2]^{1/2}$  to account for the directionality of the loadings. This approach will normally result in a conservative value as compared to FEA considering the actual loading conditions. For the models studied in this investigation, the conservatism was as high as 29%; the average value was 8%. The only unconservatism was -2%.
- 2. Additional conservatism exists in the methodology due to the effects of the combinations of the directions of the torsional and out-of-plane moments (see Section 2.5).
- 3. The method used by ANSI B31.3 to combine stresses (based on SIFs), which includes different SIFs for in-plane and out-of-plane bending, but no intensification for torsion, is apparently unconservative based on a comparison to the FEA. This unconservatism could be as high as -48.6% (model 2, C/B = 1.0, LC-4) based on the loading and models used in this study.
- 4. The methodology that best predicted the FEA results was the SQ-SUM (ADJUSTED) Combination Methodology. The corresponding methodology for use with SIFs would be:

$$S = [(i_{1} M_{1})^{2} + (a+b(C/B)^{n})* (i_{0} M_{0} + i_{0} M_{1})^{2}]^{1/2}/Z$$
 (equation 5-1)

where

a = 1.0 b = -0.187 n = 0.635  $i_{i} = 0.9/h^{2/3}$   $i_{o} = 0.75/h^{2/3}$ C/B is the ratio: M/M<sub>o</sub>

- 5. Eq. 5-1 is valid for C/B  $\leq$  1.0. For the condition when C/B > 1.0, C/B is replaced by B/C in equation 5-1.
- 6. Eq. 5-1 is valid for 90° elbows or bends with a value of  $h \le 0.86$  with no "end effects" created by attachment of the elbow to a flange or other component that would affect the ovalization of the elbow.

## Conclusions

The approach discussed above will result in a more accurate evaluation of an elbow.

# **6** REFERENCES

- 1. ASME Boiler and Pressure Vessel Code, Section III, Nuclear Power Plant Components, American Society of Mechanical Engineers, New York.
- 2. American National Standards Institute (ANSI), Code for Pressure Piping, B31.1, Power Piping, American Society of Mechanical Engineers, New York.
- 3. American National Standards Institute (ANSI), Process Piping, B31.3, Power Piping, American Society of Mechanical Engineers, New York.
- 4. W. G. Dodge and S. E. Moore, *Stress Indices and Flexibility Factors for Moment Loadings* on Elbows and Curved Pipe, Oak Ridge National Laboratory, ORNL-TM-3658, March 1972.
- 5. E. C. Rodabaugh, *End Effects on Elbows Subjected to Moment Loadings*, ORNL/Sub-2913/.7, March 1978.
- 6. A. R. C. Markl, Fatigue Tests of Piping Components, ASME Transactions 1952.
- 7. Leon Beskin, *Bending of Curved Thin Tubes*, Journal of Applied Mechanics, Transactions of the ASME, Vol. 67, New York, 1945.
- 8. Code Case N-319-2, Alternate Procedure for Evaluation of Stresses in Butt Welding Elbows in Class 1 Piping, Section III, Division 1, August 14, 1990.

# **A** TEST DATA

# **Overview of Appendix A**

The description of the testing is contained in Section 3. Table 3-1 contains a summary of the results. This appendix contains reports of the details regarding the test data for each of the four tests. Each test report contains the following:

1. Load-deflection data sheets for four conditions (=/- directions, loading, and unloading). The sheets are used to determine the linear slope of the load-deflection curves for the four loading conditions.

The data include loads, deflections, and so on. The columns identified as "modified" are for the case where adjustments are required to the data collection, such as resetting a dial gauge.

- 2. A summary plot of the load-deflection curve and the four straight lines from the load displacement data (item 1 above). This plot indicated the reasonableness of the slope of the load-deflection curves.
- 3. The fatigue test data analysis, including the displacement amplitude and number of cycles at each displacement.

TEST #:		ELBOW - A			TEST TYPE: IN-PLANE			
F = Fo + m	+ m "m" TO BE BASED ON N DATA POIN			DINTS. N = 5				
		THE VALUE	: OF "M" =	1344		Fo (LBS) =	0	
NOMINAL STRESS = $M/Z$ KSI, M=F x L,				L(IN) =	49 25	Z(IN <sup>3</sup> ) =	3.215	
DATA	MEASURED	)	MODIFIEL	)	SLOPE FOR	F	NOMINAL	
POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS	
		F		F	DATA POINT	ON 'm'		
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)	
1	0	0	0.000	0	N/A	0		
2	C.09	105	0.090	105	1.167	121	1.9	
3	C.16	203	0.160	203	1.264	215	3.3	
4	C.23	302	0.230	302	1.317	309	4.7	
5	0.3	400	0.300	40C	1.344	403	6.2	
6	C.37	500	0.370	50C	1.364	497	7.6	
7	C.45	604	0.450	604	1.364	605	9.3	
8	C 52	700	0.520	700	1.365	699	10.7	
9	0.6	800	0.600	800	1.356	807	12.4	
10	C.67	900	0.670	900	1.356	901	13.8	
11	0.75	1000	0.750	1000	1.351	1008	15.4	
12	C.83	1100	0.830	1100	1.343	1116	17.1	
13	C.91	1200	0.910	1200	1.336	1223	18.7	
14	C.98	1300	0.980	1300	1.333	1317	20.2	
15	1 06	1400	1.060	1400	1.330	1425	21.8	
16	1.15	1500	1.150	1500	1.321	1546	23.7	
17	1.23	1600	1.230	1600	1.314	1654	25.3	
18	1.32	1700	1.320	1700	1.305	1775	27.2	
19	1.41	1800	1.410	1800	1.294	18 <del>9</del> 5	29.0	
20	1.42	1780	1.420	1780	1.282	1909	29.2	
21	1.530	1900	1.530	1900	1.268	2057	31.5	
22	1.620	2000	1.620	2000	1.256	2178	33.4	
23	1 630	1970	1 630	1970	1,241	2191	33.6	
24	1.660	2000	1.660	2000	1.229	2232	34.2	
25	1.770	2100	1.770	2100	1.215	2379	36.4	

FATIGUE - LOAD DEFLECTION CURVE

POSITIVE LOAD - LOADING CONDITION

11/17/98



NOTES. 1. Positive load is down.

11/17/98

TEST #:	ELBOW - A	TEST TYPE	IN-PLANE
F = Fo + m .	"M" TO BE BASED ON N DATA PO NTS. N	J =	6
	THE VALUE OF "m" =	1457	Fo (LBS) = 300

NOMINAL STRESS = M/Z KSI, M=F x L, L(IN) = 49.25 Z(IN<sup>3</sup>) = 3.215

DATA	MEASURE	)	MODIFIED	)	SLOPE FOR		NOMINAL
POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS
		F		F	DATA POINT	ON "m"	
#	(INCHES)	(LBS)	( NCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)
1	1.77	2100	1.770	2100	1,416	2136	32 7
2	1.65	1900	1.650	1900	1,667	1962	30.0
3	1.51	1700	1.510	1700	1,535	1757	26.9
4	1.37	1500	1.3/0	1500	1,491	1553	23.8
5	1.23	1300	1.230	1300	1,469	1349	20.7
6	1.09	1100	1.090	1100	1,457	1145	17.5
	0.95	900	0.950	900	1,450	941	14.4
8	8.0	700	0.800	700	1,436	723	11.1
9	0.65	500	0.650	500	1,423	504	7.7
10	0.51	300	0.510	300	1,416	300	4.6
11	0.36	100	0.360	100	1,409	81	1.2
12	0.28	0	0.280	0	1,401	-35	-0.5
13							
14							
15							
16							
17							
18							
19							
20							
21							
22							
23							
24							
25							

## FATIGUE - LOAD DEFLECTION CURVE

## POSITIVE LOAD - UNLOADING CONDITION



NOTES

TEST #:	I	ELBOW - A			TEST TYPE:	IN-PLANE		FA	TIGUE	- LOAD D	EFLECTION	CURVE				
F = Fo + m		m" TO BE BASI THE VALUE OF	ED ON N DATA POINTS, "m" =	N = 1244		5 Fo (LBS) = 1	ס		N	IEGATIVE	LOAD - LOA		NTION		1/17/98	
NOMINAL STI	RESS = M/Z KSI, M=F x	L,	1	L(IN) = 49	.25	Z(IN <sup>3</sup> ) =	3.215									
DATA	MEASURE	n I	MODIFIED		SLOPE FOR	F	NOMINAL									
POINT					START TO	BASED	STRESS	Г								
	DEFECTION	E	DEFECTION	E	DATA POINT	ON "m"	UTREOU	-2.0	00	-1.\$	00	-1. <b>0</b> 00	-0.50	DO	0.qoo 🖌	0.500
#	(INCHES)	(LBS)	(INCHES)	(185)	(LBS/INCH)	(LBS)	(KSD									
	0.28	(203)	0.280	(100)	(EBS/INOTI) N/A	(105)	0.0									
2	0.18	-100	0.180	- 100	1 000	-124	-1.9							-		
3	0.03	-300	0.030	-300	1 211	-311	-4.8								0	
4	-0.13	-500	-0.130	-500	1.237	-510	-7.8									
5	-0.29	-700	-0.290	-700	1,244	-709	-10.9									
6	-0.45	-900	-0.450	-900	1.247	-908	-13.9	_								
7	-0.61	-1100	-0.610	-1100	1.248	-1107	-17.0	S -						-100	0	
8	-0.78	-1300	-0.780	-1300	1,242	-1319	-20.2	N N								
9	-0.95	-1500	-0.950	-1500	1.234	-1530	-23.4	õ								
10	-1.05	-1600	-1.050	-1600	1,223	-1655	-25.3	ē					r			
11	-1.15	-1700	-1.150	-1700	1.210	-1779	-27.3	No.						150	0	
12	24	-1800	-1.240	-1800	1.200	-1891	-29.0	- 1		I				-100	•	
13	-1.35	-1900	-1.350	-1900	1.187	-2028	-31.1					1				
14	-1.45	-2000	-1.450	-2000	1.175	-2152	-33.0				**					
15	-1.57	-2100	-1.570	-2100	1,160	-2302	-35.3				××					
16								-			× ,			-200	0	
17										•	<u>,</u>					
18																
19										•						
20														250	0	
21														-100	•	
22												DEFLECT	ION (INCHES)			
23											A TERT 1				ECTION	
24												OAD-DEFLECT	ON	LINCAR-DEFL	EGHON	
25																

NO1ES:

TEST #:		ELBOW - A			TEST TYPE:	IN-PLANE	
F = Fo + m <sup>·</sup> .		"m" TO BE BAS	ED ON N DATA POINT	S, N =		2	200
		THE VALUE OF	w. =	1357		F0 (LBS) =	-300
NOMINAL ST	RESS = M/Z KSI, M=F x	L,		L(IN) =	49.25	Z(IN <sup>3</sup> ) =	3.215
DATA	MEASURE	:)	MÓDIFIED	,	SLOPE FOR	F	NOMINAL
POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS
		F		F	DATA POINT	ON 'm"	
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCE)	(LBS)	(KSI)
1	-1.57	-2100	-1.570	-2100	1361	-2092	-32.0
2	-1,43	-1900	-1.430	- 1900	1357	-1902	-29.1
3	-1.28	-1700	-1 280	-1700	1357	-1698	-26.C
4	-1.14	-1500	-1.140	-1500	1353	-1508	-23.1
5	-0.99	-1300	-0.990	-1300	1356	-1304	-20.0
6	-0.84	-1100	-0.840	-1100	1360	-1101	-16.9
7	-0 69	-900	-0 690	-900	1360	-897	-13 7
8	-0.55	-700	-0.550	-700	1333	-707	-10.8
9	-0.4	-500	-0.400	-500	1333	-504	-7.7
10	-0.25	-300	-0 250	-300	1361	-300	-4.6
11	-0.100	-100	-0.100	-100	1359	-96	-1.5
12	-0.020	0	-0.020	0	1355	12	0.2
13							
14							
15							
16							
17							
18							
19							
20							
21							
22							
23							
24							
25							

## FATIGUE - LOAD DEFLECTION CURVE

## 11/17/98

## NEGATIVE LOAD - UNLOADING CONDITION



NOTES:



## FATIGUE TEST DATA ANALYSIS

11/17/98

#### **ELBOW - A** TEST #:

## TYPE: IN-PLANE

AVERAGE STIFFNESS (lbs/in) = 1351 MOMENT ARM (in)= 47.49

D (in) = 4.5

t (in) = 0.237

 $Z (in^3) = 0.0982(D^4-(D-2t)^4)/D= 3.215$ 

TEST DISPLACEMENT/CYCLE DATA:

CONDITION	DISPLACEMENT	EFFECTIVE	NOMINAL	NUMBER
#	AMPLITUDE	APPLIED	STRESS	OF TEST
	(+/-) (in.)	LOAD (lbs)	(+/-) (psi)	CYCLES
	di		S	Ni
1	0.75	1013	14,964	110
2	1.25	1689	24,940	3,645
3	1.40	1 <b>891</b>	27,932	345
4	1.55	2094	30,925	626
5	0.00	0	0	0
6	0.00	0	0	0
7	0.00	0	0	0
8	0.00	0	0	0
			TOTAL CYCLES:	4,726

EQUIVALENT NUMBER OF CYCLES, BASE	$ED ON \delta = 1.55$	INCHES
IS: $N_{eq} = SUM(\delta_i / \delta_{max})^5 * N_i = 2,080$		
FOR NOMINAL DIMENSIONS:	i = 245,000 * N $_{eq}^{(-0.2)}/S$ =	1.719
FOR Z(IN <sup>3</sup> ) = 3.215		i = <b>1.719</b>

COMMENTS:

L = 49.25 to the center of the elbow.
 Initial deflection 0.75" to 100 cycles. Change to 1.25" completed at 110 cycles.
 Deflection changed to 1.4" at 3750 cycles. Change completed at 3755 cycles.
 Deflection changed to 1.55" at 4100 cycles. Change completed at 4107 cycles.
 Failiure occured on "left" side as a crack along the centerline of the elbow at 45°.
 Moment arm to point of failure is 47.49.

TEST #:		ELBOW	- B	TEST TYPE: IN-PLANE					
F = Fo + m		"m" TO BE	"m" TO BE BASED ON N DATA POINTS, N -			7			
		THE VALUE	LOF M -	1300		F0 (LBS) -	U		
NOMINAL S	STRESS = M/Z KSI, M=F	F x L.		L(IN) =	49.25	Z(IN <sup>3</sup> ) =	3.215		
DATA	MEASURED	0	MODIFIE	0	SLOPE FOR	F	NOMINAL		
POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS		
		F		F	DATA POINT	ON "m"			
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)		
1	0	0	0.000	0	N/A	0			
2	0.1	139	0.100	139	1,390	131	2.0		
3	0.2	268	0.200	268	1,340	262	4.0		
4	0.3	412	0.300	412	1,365	392	6.0		
5	0.4	540	0.400	540	1,353	523	8.0		
6	0.5	661	0.500	661	1,329	654	10.0		
7	0.6	782	0.600	782	1,308	785	12.0		
8	0.7	918	0.700	918	1,303	916	14.0		
9	0.8	1032	0.800	1032	1,290	1046	16.0		
10	0.9	1161	0.900	1161	1,284	1177	18.0		
11	1	1282	1.000	1282	1,277	1308	20.0		
12	1.1	1395	1.100	1395	1,267	1439	22.0		
13	1.2	1509	1.200	1509	1,257	1569	24.0		
14	1.3	1607	1.300	1607	1,243	1700	26.0		
15	1.4	1705	1.400	1705	1,227	1831	28.0		
16	1 45	1759	1 450	1759	1,215	1896	29.0		

1.500

1.550 1.500

1.650

1.700

1.750

1804

1842 1887

1933

1970

2001

1,204

1,193

1,182

1,172

1,162

1,151

1962

2027

2093

2158

2223

2289

FATIGUE - LOAD DEFLECTION CURVE

11/17/98

**POSITIVE LOAD - LOADING CONDITION** 



25

16 17

18

19

20 21

22 23

24

NOTES: 1. Positive load is down.

1.5

1.55

16

1.65

1.700

1.750

1804

1842 1887

1933

1970

2001

## EPRI Licensed Material

## Test Data

11/17/98

TEST #:	ELBOW	- B	TEST TYPE:	IN-PLANE	
F – Fo + m	"m" TO BE F THE VALUE	BASED ON N DATA POINTS, N = E OF "m" = 1364		10 Fo (LBS) =	850
NOMINAL S	STRESS = M/Z KSI, M=F x I.	1 (IN) =	49 25	Z(IN <sup>3</sup> ) =	3 215
DA~A	MEASURED	MODIFIED	SLOPE FOR	F	NOMINAL

POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS
		F		F	DATA PO NT	ON "m"	
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(_BS/INCH)	(LBS)	(KSI)
1	1.75	2001	1.750	2001	N/A	2009	30.8
2	1.7	1910	1.700	1910	1.820	1941	29.7
3	1.6	1774	1.600	1774	1,491	1805	27.6
4	1.5	1645	1.500	1645	1,404	1658	25.6
5	14	1494	1.400	1494	1.416	1532	23.5
6	1.3	1365	1.300	1365	1,399	1396	21.4
7	1.2	1244	1.200	1244	1,371	1259	19.3
8	11	1092	1.100	1092	1,373	1123	17.2
9	1	957	1.000	957	1,373	986	15.1
10	C.9	835	0.900	835	1,364	850	13.0
11	C.8	699	0.800	699	1,359	714	10.9
12	C.7	555	0.700	555	1,359	577	8.8
13	C.6	434	0.600	434	1,354	441	6.8
14	0.5	305	0.500	305	1,325	304	4.7
15	C.4	177	0.400	177	1,285	168	2.6
16	C.3	56	0.300	56	1,262	32	0.5
17	C.2	0	0.240	0	1,219	-50	-0.8
18							
19							
20							
2'							
22							
23							
24							
25							

## FATIGUE - LOAD DEFLECTION CURVE

## POSITIVE LOAD - UNLOADING CONDITION



TEST LOAD-DEFLECTION
 LINEAR-DEFLECTION

NOTES:

TEST #:	I	ELBOW - B			TES⊤ ⊤YPE:	IN-PLANE	
F = Fc + m	,	'm'' TO BE BASE	ED ON N DATA POINTS.	N =		10	
	1	THE VALUE OF	"ריז	1209		Fo (LBS) =	0
NOMINAL STF	RESS = M/Z KSI, M=F x	L,	ι	.(IN) =	49.25	Z(IN <sup>3</sup> ) =	3.215
DATA	MEASURE	D	MODIFIED		SLOPE FOR	F	NOMINAL
POINT	DEFLECTION			LOAD	SIARLIO	BASED	SIRESS
	DEFECTION	F	DEFECTION	F	DATA POINT	ON "m"	OTTLEGG
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)
1	0.24	(200)	0.240	(200)	N/A	(200)	0.0
2	0.200	-65	0.200	-65	1 625	-48	-0.7
3	0.100	-186	0.100	-186	1.306	-169	-2.6
4	0	-315	0.000	-315	1.291	-290	-4.4
5	-0.1	-435	-0.100	-435	1,268	-411	-6.3
6	-0.2	-557	-0 200	-557	1,254	-532	-8.1
7	-0.3	-678	-0.300	-678	1,245	-653	-10.0
8	-0.4	-799	-0 400	-799	1,238	-774	-11.9
9	-0.5	-905	-0.500	-905	1,222	-895	-13.7
10	-0.6	-1018	-0.600	-1018	1.209	-1016	-15.6
11	-0.7	-1139	-0.700	-1139	1,202	-1137	-17.4
12	-0.8	-1245	-0.800	-1245	1.193	-1257	-19.3
13	-0.9	-1351	-0.900	-1351	1,182	-1378	-21.1
14	-1	-1472	-1.000	-1472	1,177	-1499	-23.0
15	-1.1	-1556	-1.100	-1556	1,165	-1620	-24.8
16	-1.2	-1669	-1.200	-1669	1,156	-1741	-26.7
17	-1.4	-1843	-1.400	-1843	1,137	-1983	-30.4
18	-1.5	-1949	-1.500	-1949	1,124	-2104	-32.2
19	-1.6	-2040	-1.600	-2040	1,111	-2225	-34.1
20	-1.7	-2123	-1.700	-2123	1,098	-2346	-35.9
21							
22							
23							
24							
25							



11/17/98

FATIGUE - LOAD DEFLECTION CURVE

**NEGATIVE LOAD - LOADING CONDITION** 

NOTES.

11/17/98

500

1000

-1500

-2000

-2500

0.000

TEST #:		ELBOW - E	3		TEST TYPE:	IN-PLANE		F	ATIO	GUE - I		DEFLECT	ION CURVI	Ξ					
F = Fo + m. :		"m" TO BE BAS THE VALUE O	SED ON N DATA POINTS F "m ' =	S, N = 1293	:	2 Fo (I RS) =	-912			NE	GATIVI	E LOAD -	UNLOADIN	IG CON		N			
NOMINAL ST	RESS – M/Z KSI, M–F >	< L.	I	L(IN) – 4	49.25	Z(IN <sup>3</sup> ) =	3.215												
DATA	MEASURE	ED	MODIELED	1	SLOPE FOR	F	NOMINAL		r							-			
POINT	DEFLECTION		DEFLECTION		START TO	BASED	STRESS												
1 0.11	DEFECTION	F	DEFECOTION	F	DATA POINT	ON "m"	UNLEGO												
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)												
1	-17	-2123	-1 700	-2123	N/A	-2075	-31.8									_			
2	-1.6	-1957	-1 600	-1957	1293	- 1946	-29.8	-1.8	00	-1.6	600	-1.400	-1.200	-1.000	0	-0.800	-0.\$00		-0.400
3	-1.5	-1821	-1.500	-1821	1289	-1817	-27.8												
4	-1.4	-1692	-1.400	-1692	1293	- 1688	-25.8												
5	-1.3	-1563	-1.300	-1563	1300	- 1558	-23.9											~	
6	-1.2	-1442	-1.200	-1442	1317	- 1429	-21.9												
7	-1.1	-1313	-1.100	-1313	1324	-1300	-19.9	S											
8	-1	-1177	-1.000	-1177	1325	-1171	-17.9	Ś.											
9	-0.9	-1056	-0.900	-1056	1440	- 1041	-15.9	6							and the second				
10	-0.8	-912	-0.800	-912	1314	-912	-14.0	ŏ						-					
11	0.700	799	0.700	799	1307	783	12.0	ð					1000	Para la constante da la constante d					
12	-0.600	-655	-0.600	-655	1308	-654	-10.0	2					and the second						
13	-0.500	-526	-0.500	-526	1308	-524	-8.0					/	•						
14	-0.400	-398	-0.400	-398	1307	-395	-6.1					-							
15	-0.300	-277	-0.300	-277	1304	-266	-4.1				~								
16	-0.200	-155	-0.200	-155	1300	-137	-2.1				r								_
17	0.100	34	0.100	34	1296	7	0.1			*									
18	-0.060	0	-0.060	0	1290	44	0.7												
19	0.000	78	0.000	78	1286	122	1.9												
20												-							
21														DEFL	ECTION	INCHES)			
22																· -··/			
23																			
24													_A_ TES				1		
25																		1241-01	

NOTES:



## FATIGUE TEST DATA ANALYSIS

11/17/98

#### **ELBOW - B** TEST #:

## TYPE: IN-PLANE

AVERAGE STIFFNESS (lbs/in) = 1293 MOMENT ARM (in)= 47.49

D (in) = 4.5	t (in) = 0.237	$Z (in^3) = 0.0982(D^4-(D-2t)^4)/D= 3.215$
--------------	----------------	--

TEST DISPLACEMENT/CYCLE DATA:

CONDITION	DISPLACEMENT	EFFECTIVE	NOMINAL	NUMBER
#	AMPLITUDE	APPLIED	STRESS	OF TEST
	(+/-) (in.)	LOAD (lbs)	(+/-) (psi)	CYCLES
	di		S	Ni
1	1.45	1875	27,699	2,685
2	1.50	1940	28,654	624
3	0.00	0	0	0
4	0.00	0	0	0
5	0.00	0	0	0
6	0.00	0	0	0
7	0.00	0	0	0
8	0.00	0	0	0
			TOTAL CYCLES:	3,309

EQUIVALENT NUMBER OF CYCLES, BAS	SED ON $\delta = 1.5$	INCHES
IS: $N_{eq} = SUM(\delta_i/\delta_{max})^5 * N_i = 2,890$		
FOR NOMINAL DIMENSIONS:	i = 245,000 * N $_{eq}^{(-0.2)}/S$ =	1.737
FOR Z(IN <sup>3</sup> ) = 3.215		i = <b>1.737</b>

COMMENTS:

1. L = 49.25 to the center of the elbow.

2. 2 cycles to measure deflection.

2. 2 cycles to measure denection.
 3. Deflection cahnged to 1.5 inches at 2680 cycles. Change complete at 2685 cycles.
 4. Failure occured on "left" side as a crack along the centerline of the elbow at 45°.
 5. Moment arm to point 0f failure is 47.49

ELBOW - C		TEST TYPE: OUT-OF-P	LANE
"m" TO BE BASED ON N DATA	POINTS, N =	5	
THE VALUE OF "m" =	1155	Fo (LBS) =	= 0

NOMINAL STRESS = M/Z KSI, M=F x L, L(IN) = 49.25  $Z(IN^2) =$ 3.215

DATA	MEASURED	)	MODIFIED	)	SLOPE FOR	F	NOMINAL
POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS
		F		F	DATA POINT	ON "m"	
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)
1	0	0	0.000	0	N/A	0	
2	0.1	111	0.100	111	1,110	116	1.8
3	0.2	225	0.200	225	1,125	231	3.5
4	0.3	346	0.300	346	1,152	347	5.3
5	0.4	460	0.400	460	1,155	462	7.1
6	0.5	581	0.500	581	1,164	578	8.8
7	0.6	709	0.600	709	1,179	693	10.6
8	0.7	815	0.700	815	1,176	808	12.4
9	0.8	929	0.800	929	1,172	924	14.2
10	0.9	1042	0.900	1042	1,168	1040	15.9
11	1	1148	1.000	1148	1,160	1155	17.7
12	1.1	1254	1.100	1254	1,152	1271	19.5
13	1.2	1375	1.200	1375	1,149	1386	21.2
14	1.3	1481	1.300	1481	1,145	1502	23.0
15	1.4	1595	1.400	1595	1,142	1617	24.8
16	1.5	1678	1.500	1678	1,133	1733	26.5
17	1.6	1799	1.600	1799	1,128	1848	28.3
18	1.7	1898	1.700	1898	1,123	1964	30.1
19	1.8	1996	1.800	1996	1,116	2079	31.8
20	1.9	2087	1.900	2087	1,109	2195	33.6
21	2	2178	2.000	2178	1,101	2310	35.4
22	2.1	2246	2.100	2246	1,090	2426	37.2
23							
24							
25							



11/18/98

2.500

FATIGUE - LOAD DEFLECTION CURVE

0.500

POSITIVE LOAD - LOADING CONDITION

DEFLECTION (INCHES)

1.500

2.000

1.000

Test Data

TEST #:

F = Fo + m

NOTES: 1. Positive load is down.

3000

01 0.000

A-15

11/18/98

TEST #:	ELBOW - C		TEST TYPE: OUT-OF-PLANE	
F = Fo - m	"m" TO BE BASED ON N DA	TA POINTS, N =	15	
	THE VALUE OF "m" -	1225	Fo (LBS) - 1050	
NOMINAL STRESS = M/Z KSI,	M=F x L.	L(IN) = 49.2	5 Z(IN <sup>3</sup> ) = 3.215	

DATA	MEASURED		MODIFIED		SLOPE FOR	F	NOMINAL
POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS
		F		F	DATA POINT	ON "m"	
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)
1	2.1	2246	2.100	2246	N/A	2153	33.0
2	2	2079	2.000	2079	1,670	2030	31.1
3	1.9	1958	1.900	1958	1,440	1908	29.2
4	1.8	1814	1.800	1814	1,417	1785	27.3
5	17	1593	1.700	1693	1,371	1663	25.5
6	16	1572	1.600	1572	1,335	1540	23.6
7	1.5	1444	1.500	1444	1,316	1418	21.7
8	1 4	1307	1.400	1307	1,313	1295	19.8
9	1.3	1179	1.300	1179	1,309	1173	18.0
10	1.2	1058	1.200	1058	1,302	1050	16.1
11	1.1	929	1.100	929	1,297	928	14.2
12	1	808	1.000	808	1,291	805	12.3
13	0.9	697	0.900	697	1,282	683	10.5
14	0.8	566	0.800	566	1,216	560	8.6
15	0.7	452	0.700	452	1,225	438	6.7
16	0.6	331	0.600	331	1,212	315	4.8
17	0.5	217	0.500	217	1,195	193	2.9
18	0.4	104	0.400	104	1,181	70	1.1
19	0 29	0	0 290	0	1,148	-65	-10
20							
21							
22							
23							
24							
25							

FATIGUE - LOAD DEFLECTION CURVE

## POSITIVE LOAD - UNLOADING CONDITION



NOTES:

-0.400

-0.500

-0.6

0.700

-0.800

-1.000

-1.100

-1.300

-1.400

-1.600

-1.700

-1.900

-2.000

-2 100

-1.8

-15

-1.2

-0.9

-750 -826

-947

1045

-1129

-1235

-1341

-1424

-1522

-1621

-1704

-1787

-1870

-1954

-2037

-2113

-2181 -2256

TEST #:		ELBOW - C	;	TEST TYPE: OUT-OF-PLANE				
F = Fo + m "m" TO BE BASED ON N DATA POINTS, N = THE VALUE OF 'm' =				s, N = 1053		10 Го (LBS) =	0	
NOMINAL ST	TRESS = M/Z KSI, M=F :	κ L,		L(IN) =	49.25	∠(IN³) =	3.215	
DATA	MEASUR	ED	MODIFIED	)	SLOPE FOR	F	NOMINAL	
POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS	
		F		F	DATA POINT	ON "m"		
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)	
1	0.29	0	C.290	0	N/A	0		
2	0.200	-107	C 200	-107	1,189	-95		
3	0.100	-213	0.100	-213	1,120	-200		
4	0	-326	C.000	-326	1,117	-305		
5	-0.100	-417	-C 100	-417	1,074	-411		
6	-0.200	-531	-0.200	-531	1,071	-516	-	
7	-0.3	-637	-0.300	-637	1,069	-621		

-C.400

-0.500

-C.600

C.700

-0.800

-C.900

-1.000

-1.100

-1.200

-1.300

-1 500

-1.600

-1.700

-1 800

-1.900

-2.000

-2 100

-750

-826

-947

1045

-1129 -1235

-1341

-1424

-1522

-1621 -1704

-1787

-1870

-1954

-2037

-2113

-2181

-2256

1,074

1,055

1,053

1,049

1,039

1,033

1,031

1,024

1,018

1,014

1,000

993

985

977

969

960

951

-727

-832

-938

1043 -1148

-1254

-1359

-1464

-1570

-1675 -1780

-1886

-1991

-2096

-2202

-2307

-2412

-2518

FATIGUE - LOAD DEFLECTION CURVE

#### 11/18/98

NEGATIVE LOAD - LOADING CONDITION



25 NOTES:

8

9

10

11

12

13

14

15

16

17

18

19

20

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23

24

TEST #:		ELBOW - C	•	TEST TYPE: OUT-OF-PLANE				FA1	IGUE - LOAD	
F = Fo - m		"m" TO BE BAS	ED ON N DATA POINT	S, N =		7		NEGA		
		THE VALUE OF	"'m'' =	1093		Fo (LBS) =	-387			
NOMINAL ST	RESS = M/Z KSI, M=F :	¢L,		L(IN) =	49 25	Z(IN <sup>3</sup> ) =	3.215			
0.174			MADIFICO					_		
	MLASURE		MODIFIED		SLOPE FOR		NUMINAL			
POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS			
		F		F	DATA POINT	ON "m"				
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KS)			
1	-2.1	-2256	-2.100	-2256	N/A	-2246	-34.4	-2.500		
2	-2	-2128	-2.000	-2128	1280	-2136	-32.7			
3	-1.8	-1923	-1.800	-1923	1098	-1918	-29.4			
4	-1.6	-1704	-1.600	-1704	1090	-1699	-26.0			
5	-1.4	-1484	-1.400	-1484	1091	-1480	-22.7			
6	-1.2	-1265	-1.200	-1265	1092	•1262	-19.3	ŝ		
7	-1	-1045	-1.000	-1045	1093	-1043	-16.0	Q I		
8	-0.8	-818	-0.800	-818	1097	-824	-12.6	- 5		
9	-06	-599	-0 600	-599	1099	-606	-93	ē,		
10	-0.4	-387	-0.400	-387	1098	-387	-5.9	Q I		
11	0.200	175	0.200	175	1095	168	2.6	q		
12	0.000	51	0.000	51	1095	50	8.0			
13										
14										
15										
16								-		
17									_	
18									ŧ.	
19										
20										
21										
22										
23										
24										
25										

## ATIGUE - LOAD DEFLECTION CURVE

## 11/18/98

## NEGATIVE LOAD - UNLOADING CONDITION





SUMMARY LOAD-DEFLECTION

EPRI Licensed Material

## FATIGUE TEST DATA ANALYSIS

11/18/98

#### **ELBOW - C** TEST #:

## TYPE: OUT-OF-PLANE

AVERAGE STIFFNESS (lbs/in) = 1132 MOMENT ARM (in)= 49.25

D (in) = 4.5

t (in) = 0.237

 $Z (in^3) = 0.0982(D^4-(D-2t)^4)/D= 3.215$ 

TEST DISPLACEMENT/CYCLE DATA:

CONDITION	DISPLACEMENT	EFFECTIVE	NOMINAL	NUMBER
#	AMPLITUDE	APPLIED	STRESS	OF TEST
	(+/-) (in.)	LOAD (lbs)	(+/-) (psi)	CYCLES
	di		S	Ni
1	2.15	2433	37,268	3,820
2	2.20	2490	38,135	138
3	0.00	0	0	0
4	0.00	0	0	0
5	0.00	0	0	0
6	0.00	0	0	0
7	0.00	0	0	0
8	0.00	0	0	0
			TOTAL CYCLES:	3,958

EQUIVALENT NUMBER OF CYCLES, BASED ON	δ = 2.2	INCHES
IS: $N_{eq} = SUM(\delta_i/\delta_{max})^5 * N_i = 3,543$		
FOR NOMINAL DIMENSIONS: i = 24	5,000 * N <sub>eq</sub> <sup>(-0.2)</sup> /S =	1.253
FOR Z(IN <sup>3</sup> ) = 3.215		i = 1.253

COMMENTS:

- L = 49.25 to the center of the elbow.
   Deflection started at 1.75". Changed to 2.1" in 8 cycles.
   Loading rate: 16 cycles/min.

At 3665 changed deflection to 2.5. Bottomed out at 3675. Restarted and reached 2.2 at 3690. Assume changed to 2.2 at 3665.

TEST #:		ELBOW - D TEST TYPE: OUT-OF-PLANE					ANE
F = Fo + m		"m" TO BE B	ASED ON N DATA PO	INTS, N =		10	
		OF "m" =	1123		Fo (LBS) =	0	
NOMINAL S	TRESS = M/Z KSI, M=F	хL,		L( N) =	49.25	Z(IN <sup>3</sup> ) =	3.215
	MEASURED		MODIFIEL	1	SLOPE FOR	F	NOMINAL
POINT		LOAD	DEFLECTION		START TO	BASED	STRESS
		F		F	DATA POINT	ON "m"	
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)
1	0.00	0	0.000	0	N/A	0	(**=*/
2	0.10	109	0.100	109	1,090	112	1.7
3	0.20	230	0.200	230	1,150	225	3.4
4	0.30	344	0.300	344	1,153	337	5.2
5	0.40	457	0.400	457	1,149	449	6.9
6	0.50	556	0.500	556	1,125	562	8.6
7	0.60	677	0.600	677	1,126	674	10.3
8	0.80	904	0.800	904	1,128	898	13.8
9	1.00	1131	1.000	1131	1,129	1123	17.2
10	1.20	1343	1.200	1343	1,123	1348	20.6
11	1.40	1570	1.400	1570	1,121	1572	24.1
12	1.60	1789	1.600	1789	1,119	1797	27.5
13	1.70	1888	1.700	1888	1,115	1909	29.2
14	1.80	2001	1.800	2001	1,112	2021	31.0
15	1.90	2092	1.900	2092	1,108	2134	32.7
16	2.00	2206	2.000	2206	1,105	2246	34.4
17	2.10	2289	2.100	2289	1,100	2358	36.1
18	2.20	2372	2.200	2372	1,093	2471	37.8
19	2.30	2463	2.300	2463	1,086	2583	39.6
20	2.40	2501	2.400	2501	1,074	2695	41.3
21							
22							
23							
24							
25							



FATIGUE - LOAD DEFLECTION CURVE

**POSITIVE LOAD - LOADING CONDITION** 

NOTES: 1. Positive load is down.

LOAD (POUNDS)

## Test Data

11/19/98

TEST #:	ELBOW - D	TEST TYPE:	TEST TYPE: OUT-OF-PLANE			
F = Fc + m	"m" TO BE BASED ON N DATA PO THE VALUE OF "m" =		12 Fo (LBS) =	300		
NOMINAL STRESS = M/Z KSI, M=	FxL,	L(IN) = 49	9.25	Z(IN <sup>3</sup> ) =	3.215	

DATA	MEASURED	) (	MÓDIFIED	)	SLOPE FOR	F	NÓMINAL
POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS
		F		F	DATA POINT	ON "m"	
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)
1	2.40	2501	2.400	2501	N/A	2450	37.5
2	2.20	2221	2.200	2221	· ,400	2211	33.9
3	2.00	1956	2.000	1956	1,362	19/2	30.2
4	1.80	1721	1.800	1721	1,302	1733	26.5
5	1.60	<b>*</b> 472	1.600	1472	°,279	1494	22.9
6	1.40	1214	1.400	1214	1,2/4	1255	19.2
7	1.20	980	1.200	980	<sup>2</sup> ,261	1017	15.6
8	1.00	737	1.000	737	1,252	778	11.9
ĝ	0.80	510	0.800	510	1,240	539	8.3
10	D.60	291	0.600	291	<sup>-</sup> ,226	300	4.6
11	0.40	79	0.400	79	1,210	61	0.9
12	0.30	0	0.300	0	194,194	-58	-0.9
13							
14							
15							
16							
17							
18							
19							
20							
21							
22							
23							
24							
25							

FATIGUE - LOAD DEFLECTION CURVE

## POSITIVE LOAD - UNLOADING CONDITION



DEFLECTION (INCHES)

NOTES:
TEST #:			ELBOW - D	)		TEST TYPE: OUT-OF-PLANE			
	F = Fo + m		"m" TO BE BAS THE VALUE OF	ED ON N DATA POINTS	6, N = 966		10 Fo (LBS) - 0		
	NOMINAL ST	RESS = M/Z KSI, M=F ;	×L,		L(IN) =	49.25	Z(IN <sup>3</sup> ) =	3.215	
		MEASURI	ED	MODIEIEI	ר	SLOPE FOR	F		
	POINT	DEFLECTION		DEFLECTION		START TO	BASED	STRESS	
			F		F	DATA POINT	ON "m'		
	#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)	
	1	0.300	0	0.300	0	N/A	0		
	2	0.100	-223	0.100	-223	1,115	-193	-	
	3	-0.100	-427	-0.100	-427	1,068	-386	-	
	4	-0.300	-616	-0.300	-616	1.026	-580	-	
	5	-0.500	-813	-0.500	-813	1.010	-773	-1	
	6	-0.700	-1002	-0.700	-1002	996	-966	-1	
	7	-0.900	-1192	-0.900	-1192	986	-1159	-1	
	8	-1.100	-1381	-1.100	-1381	978	-1352	-2	
	9	-1.300	-1570	-1.300	-1570	973	-1546	-2	
	10	-1.400	-1653	-1.400	-1653	966	-1642	-2	
	11	-1.500	-1752	-1.500	-1752	963	-1739	-2	
	12	1 600	1835	1.600	1835	959	1835	2	
	13	-1.700	-1926	-1.700	-1926	955	-1932	-2	
	14	-1.800	-2024	-1.800	-2024	953	-2029	-3	
	15	-1 900	-2100	-1.900	-2100	948	-2125	-3	
	16	-2.000	-2191	-2.000	-2191	945	-2222	-3	
	17	-2.100	-2342	-2.100	-2342	948	-2319	-3	

FATIGUE - LOAD DEFLECTION CURVE

11/19/98

#### **NEGATIVE LOAD - LOADING CONDITION**



NOTES:

18 19 20

21 22 23

2∠

25

A-23

## 40 -2.000 -1.\$00 -0.500 -1.000 500 1000 1500 2000 7 -2500

DEFLECTION (INCHES)



EPRI	Licensed	Material

-2.\$00

LOAD (POUNDS)

# FATIGUE - LOAD DEFLECTION CURVE

#### 11/19/98

500

0.000.0

#### ELBOW - D TEST TYPE: OUT-OF-PLANE TEST #: 'm" TO BE BASED ON N DATA POINTS. N = THE VALUE OF "m" = 1 F – Fo + m 12 1128 Fo (LBS) = -321 NOMINAL STRESS = M/Z KSI, M=F x L, L(IN) = 49.25 Z(IN<sup>3</sup>) = 3.215

DATA	MEASURE	D	MODIFIED		SLOPE FOR	F	NOMINAL
POINT	DEFLECTION	LOAD	DEF_ECTION	LOAD	START TO	BASED	STRESS
		F		F	DATA POINT	ON "m"	
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)
1	-2.10	-2342	-2.100	-2342	N/A	-2239	-34.3
2	-2.00	-2100	-2.000	-2100	2420	-2127	-32.6
3	-1.80	-1873	-1.800	-1873	1502	-1901	-29.1
4	-1.60	-1646	-1.600	-1646	1331	-1675	-25.7
5	-1.40	-1419	-1.400	-1419	1260	-1449	-22.2
6	-1.20	-1184	-1.200	-1184	1229	-1224	-18.7
7	-1 00	-965	-1 000	-965	1204	-998	-15 3
8	-0.80	-745	-0.800	-745	1184	-772	-11.8
9	-0.60	-526	-0.600	-526	1169	-547	-8.4
10	-0.40	-321	-0.400	-321	1154	-321	-4.9
11	-0.20	-109	-0.200	-109	1141	-95	-1.5
12	-0.08	0	-0.080	0	1128	40	0.6
13							
14							
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NOTES

Test Data

NEGATIVE LOAD - UNLOADING CONDITION



### FATIGUE TEST DATA ANALYSIS

11/19/98

## TEST #: ELBOW - D

# TYPE: OUT-OF-PLANE

AVERAGE STIFFNESS (lbs/in) = 1103 MOMENT ARM (in)= 49.25

D (in) = 4.5 t (in) = 0.237 Z (in<sup>3</sup>) = C

Z (in<sup>3</sup>) = 0.0982(D <sup>4</sup>-(D-2t)<sup>4</sup>)/D= 3.215

TEST DISPLACEMENT/CYCLE DATA:

CONDITION	DISPLACEMENT	EFFECTIVE	NOMINAL	NUMBER
#	AMPLITUDE	APPLIED	STRESS	OF TEST
	(+/-) (in.)	LOAD (lbs)	(+/-) (psi)	CYCLES
	di		S	Ni
1	2.30	2537	38,857	2,879
2	0.00	0	0	0
3	0.00	0	0	0
4	0.00	0	0	0
5	0.00	0	0	0
6	0.00	0	0	0
7	0.00	0	0	0
8	0.00	0	0	0
			TOTAL CYCLES:	2,879

ON $\delta =$	2.3	INCHES
i = 245,000 * N <sub>eq</sub>	<sup>(-0.2)</sup> /S =	1.282
		i = 1.282
	) ΟΝ δ = i = 245,000 * N <sub>eq</sub>	$\delta = 2.3$ i = 245,000 * N <sub>eq</sub> <sup>(-0.2)</sup> /S =

COMMENTS:

1. L = 49.25 to the center of the elbow.

TEST #:		ELBOW - E		TEST TYPE: COMBINED-45			E.	ATIGUE - LO	AD DEFLEC	TION CURVE	E		11/19/98			
F = Fo + m		"m" TO BE B THF VAI UF	ASED ON N DATA PO OF "m" =	DINTS, N = 1068		7 Fo (I BS) =	0			POSIT	TIVE LOAD -		ONDITION			
NOMINAL S	TRESS = M/Z KSI, M=F	F×L.		L(IN) = 4	49.25	Z(IN <sup>5</sup> ) =	3.215									
	MEAGURER		NODIFIE		01.005 500	-	NOMEN	1	3000							
DATA	MEASUREL	, 104D			SLOPE FOR		NUMINAL									
POINT	DEFLECTION	LOAD	DEFLECTION		DATA DOINT	BASED	SIRESS									
							(KSI)									
<i>#</i>	(INCILS)	(LD3)	0.000	(103)	(LBG/INCH) N/A	(LUG)	(KOI)		2500							-
2	0.00	226	0.200	226	1 130	214	33									
3	0.40	446	0.400	446	1,115	427	6.5									
4	0.60	659	0.600	659	1,099	641	9.8		2000						- A - A - A -	
5	0.80	877	0.800	877	1,094	854	13.1		2000							
6	1.00	1082	1.000	1082	1,082	1068	16.4								•	
7	1.20	1279	1.200	1279	1,068	1281	19.6	ତ								
8	1.40	1475	1.400	1475	1,055	1495	22.9	Ð	1500							
9	1.50	1581	1.500	1581	1,049	1602	24.5	8								
10	1.60	1684	1.600	1684	1,045	1709	26.2	ē								
11	1.70	1748	1.700	1748	1,034	1815	27.8	P P								
12	1.80	1846	1.800	1846	1,026	1922	29.4	9	1000							
13	1.90	1914	1.900	1914	1,015	2029	31.1									
14	2.00	1982	2.000	1982	1,002	2136	32.7									
15	2.10	2050	2.100	2050	988	2243	34.3						-			
16	2.10	2010	2.100	2010	974	2243	34.3		500		1000					
1/	2.20	2088	2.200	2088	960	2349	36.0									
18	2.30	21.1	2.306	2111	942	2456	37.6				-					
20										4	-					
20									0 🖬	<i></i>				l		
22									0.00	D	0.500	1.0	00	1.500	2.000	2.500
23													DEFLECTION (	INCHES)		
24													,			
25											_ <b>←</b> _TE	ST LOAD-DEFLEG	TION — L	INEAR-DEFLECTION	COLLAPSE LINE	

NOTES: 1. Positive load is down

## FATIGUE - LOAD DEFLECTION CURVE

POSITIVE LOAD - UNLOADING CONDITION

#### 11/19/98

#### TEST #: ELBOW - E TEST TYPE: COMBINED-45 F = Fc + m "m" TO BE BASED ON N DATA POINTS, N = THF VAL UF OF "m" = 10 100 10 Fo (LBS) = 100 NOMINAL STRESS = M/Z KSI, M=F x L, L(IN) = 49.25 Z(IN<sup>3</sup>) = 3.215

DATA	MEASURED		MOD FIED	MOD FIED		F	NOMINAL
POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS
		F		F	DATA POINT	ON "m"	
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)
1	2.30	2111	2.300	2111	NA	2091	32.0
2	2.10	1869	2.100	1869	1,210	1870	28 6
3	1.90	1626	1.900	1626	1,213	1649	25.3
4	1.70	1399	1.700	1399	1,189	1427	21.9
5	1.50	1180	1.500	1180	1,166	1206	18.5
6	1.30	945	1.300	945	1,161	985	15.1
7	1.10	733	1.100	733	1,148	764	11 7
8	0.90	529	0.900	529	1,132	542	8.3
9	0.70	317	0.700	317	1,120	321	4.9
10	0.50	<sup>-</sup> 20	0.500	120	1,106	100	1.5
11	0.39	0	0.390	0	1,099	-22	-0.3
12							
13							
14							
15							
16							
17							
18							
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22							
23							
24							
25							

2500 2000 1500 1000 500 500 0,000 1,000 1,500 2,000 2,000 2,000 2,000 2,000 2,000 2,000

NOTES:

## Test Data

TEST #:	ELBOW - E		TEST	TYPE: COMBINI	ED-45
F = Fo + m	"m" TO BE BASED ON N DATA THE VALUE OF "m" =	POINTS, N =	386	10 Fo (LB	(S) = 0
NOMINAL STRESS = M/Z	KSI, M=F x L,	L(IN) =	49.25	Z(IN <sup>3</sup> ) =	3.215

DATA	MEASURE	D	MODIFIED		SLOPE FOR	F	NOMINAL
POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS
		F		F	DATA POINT	ON ''m''	
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)
1	C.390	0	C.390	C	N/A	0	0.0
2	0.300	-68	C.300	-68	756	-80	-1.2
3	C.100	-257	C.100	-257	896	-257	-3.9
4	-0.100	-431	-C.100	-431	891	-435	-6.7
5	-0.300	-620	-0.300	-620	903	-612	-9.4
6	-0.500	-825	-C.500	-825	925	-790	-12.1
7	-0.700	-991	-C.700	-991	921	-968	-14.8
8	-0.900	-1158	-C.900	-1158	912	-1145	-17.5
9	-1.100	-1317	-1.100	-1317	900	-1323	-20.3
10	-1.300	-1476	-1.300	-1476	888	-1500	-23.0
11	-1.500	-1642	-1.500	-1642	878	-1678	-25.7
12	-1.600	-1710	-1.600	-1710	870	-1766	-27.1
13	-1.700	-1793	-1.700	-1793	864	-1855	-28.4
14	-1 800	-1862	-1 800	-1862	857	-1944	-29 8
15	-1.900	-1930	-1.900	-1930	851	-2033	-31.1
16	-2.000	-2005	-2.000	-2005	845	-2121	-32 5
17	-2.190	-2066	-2.100	-2066	838	-2210	-33.9
18	-2.200	-2142	-2.200	-2142	833	-2299	-35.2
19							
20							
21							
22							
23							
24							
25							

#### FATIGUE - LOAD DEFLECTION CURVE

#### **NEGATIVE LOAD - LOADING CONDITION**



11/19/98

NOTES

TEST TYPE: COMBINED-45

### Test Data

F = Fo + m	, 1	m" TO BE BAS THE VALJE OF	ED ON N DATA POINT "m" =	S, N = 1012		12 Fo (LBS) = -280		
NOMINAL ST	IRESS = M/Z KSI, M=F x	ί,		l (IN) =	49.25	∠(IN <sup>2</sup> ) =	3.215	
	MEASL PE		MODIELEE	)		F	NOMINAL	
POINT					START TO	BASED	STRESS	
FOINT	DEFLECTION	F	DEFECTION	E		ON "m"	0111200	
#	(INCHES)	(188)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSD	
1	-2.20	-2142	-2 200	-2142	N/A	-2102	-32.2	
2	-2.00	-1907	-2 000	-1907	175	-1900	-29,1	
3	-1.80	-1688	-1 800	-1688	135	-1697	-26.0	
4	-1.60	-1476	-1 600	-1476	<sup>-</sup> 109	- 1495	-22.9	
5	-1.40	-1271	-1 400	-1271	1087	-1292	-19.8	
6	-1.20	-1067	-1 200	-1067	1071	-1090	-16.7	
7	-1.00	-863	-1 000	-863	ŕ 060	-887	-13.6	
8	-0.80	-681	-0 800	-681	·043	-685	-10.5	
9	-0.60	-484	-0 600	-484	1031	-482	-7.4	
10	0.40	280	0 400	280	1024	280	4.3	
11	-0.20	-83	-0 200	-83	1019	-78	-1.2	
12	-0.10	0	-0 100	0	1012	24	0.4	
13								
14								
15								
16								
17								
18								
19								
20								
21								
22								
23								
24								
25	1				1	1	1	



#### 11/19/98

#### NEGATIVE LOAD - UNLOADING CONDITION



NOTES:

TEST #:

ELBOW - E

A-29





### FATIGUE TEST DATA ANALYSIS

11/19/98

#### **ELBOW - E** TEST #:

TYPE: COMBINED-45

AVERAGE STIFFNESS (lbs/in) = 1018 MOMENT ARM (in)= 49.25

D (in) = 4.5

t (in) = 0.237

 $Z (in^{3}) = 0.0982(D^{4}-(D-2t)^{4})/D= 3.215$ 

TEST DISPLACEMENT/CYCLE DATA:

CONDITION	DISPLACEMENT	EFFECTIVE	NOMINAL	NUMBER
#	AMPLITUDE	APPLIED	STRESS	OF TEST
	(+/-) (in.)	LOAD (lbs)	(+/-) (psi)	CYCLES
	di		S	Ni
1	2.05	2088	31,981	3,344
2	0.00	0	0	0
3	0.00	0	0	0
4	0.00	0	0	0
5	0.00	0	0	0
6	0.00	0	0	0
7	0.00	0	0	0
8	0.00	0	0	0
			TOTAL CYCLES:	3,344

EQUIVALENT NUMBER OF CYCLES, BASED	ΟN δ =	2.05 l	NCHES
IS: $N_{eq} = SUM(\delta_i / \delta_{max})^5 * N_i = 3,344$			
FOR NOMINAL DIMENSIONS:	i = 245,000 * N eq <sup>(-0</sup>	<sup>2)</sup> /S =	1.512
FOR Z(IN <sup>3</sup> ) = 3.215		i= 1	1.512

COMMENTS:

L = 49.25 to the center of the elbow.
 Tests stopped at about 2600 cycles. Then restarted.

11/19/98

2.500

## Test Data

PF-PRI         TENEBROTING TATE         Definition           DOTING STRESS + UZ RIM FLA         Link • d.2         Definition         Definition	TEST #:	#: ELBOW - F		TEST TYPE: COMBINED-45			FATIGUE - LOAD DEFLECTION CURVE						11				
	F = Fo + m		"m" TO BE THF VAI UF	BASED ON N DATA PO = OF "m" =	INTS, N = 1114		10 Fo (I BS) =	0			POSIT	IVE LOAD - L					
DATA         MEASURED         MODIFIED         SLOPE FOR         F         NOMINAL           POINT         DEFLECTION         LOAD         PERLECTION         ILAD         DEFLECTION         ILAD         STREST           #         (INCHES)         (LBS)         (ILAS)         ILABPIONT         BASED         NOMINAL           2         0.020         2.37         0.000         0         ILASPIONT         (KS)           3         0.401         475         0.4000         797         1.198         248         6.16           4         0.60         661         0.6000         1.911         1.178         8.91         10.271           7         1.20         1.955         1.203         1.935         1.133         1.937         22.16           9         1.50         1.600         1.937         1.133         1.937         22.13         1.900	NOMINAL S	FRESS = M/Z KSI, M=I	F x L,		L(IN) = 4	49.25	Z(IN <sup>3</sup> ) =	3.215	30/	00							
DOINT         DEFLECTION         LOAD         DEFLECTION         LOAD         START YO DATA POINT         BSIRESS ON 'm'         CRESS           #         (INCHES)         (LGS)         (ILBS)         START YO DATA POINT         BSIRESS         STRESS           1         0.00         0         0.000         0         NM         0           2         0.20         2.37         0.200         2.37         1.185         2.3.34           3         0.40         4/4         0.60         691         1.1.56         602           5         0.60         911         1.1.35         891         13.7           7         1.20         1.365         1.1.33         13.37         20.23           9         1.50         1.600         1.720         1.684         22.60           10         1.60         1.700         1.893         1.1.17         1.894         22.00           11         1.70         1.800         1.900         1.005         22.90         1.005         22.91           12         1.60         1.900         1.005         22.84         3.93         3.93           19         2.00         2.000         2.000 <t< td=""><td></td><td>MEASURED</td><td>)</td><td>MODIEIEC</td><td></td><td>SLOPE FOR</td><td>F</td><td>ΝΟΜΙΝΑΙ</td><td>000</td><td></td><td></td><td></td><td></td><td></td><td></td><td></td><td></td></t<>		MEASURED	)	MODIEIEC		SLOPE FOR	F	ΝΟΜΙΝΑΙ	000								
Point         Del Lecitori         Del Lecitori <thdel lecitori<="" th="">         Del Lecitori</thdel>	BOINT					START TO	BASED	STDESS									
#         (INCHES)         (LBS)         (LBS)         (LBS)         (KB)           1         000         0         000         0<	POINT	DEFLECTION	F	DEFLECTION	F	DATA POINT	ON "m"	SIRESS									
1       0.00       0       0.000       0       NA4       0         2       0.00       23       0.40       472       0.400       473       1.188       23       3         4       0.66       661       0.600       691       1.188       666       10.2         5       0.66       911       0.188       691       10.2         6       1.00       1145       1.100       1133       1337       20.5         8       1.40       1577       1.122       1671       25.6       171       1.88       691       157         8       1.40       1577       1.122       1671       25.6       1.00       1781       1.114       1784       27.3         11       1.70       1880       1.07       1884       22.03       1.014       22.66       30.7         12       1.80       1970       1.800       22.26       34.1       1.55       1.50       2.000       2.000       2.001       2.266       3.9.3         15       2.210       2.200       2.220       1.032       2.461       3.58       1.50       1.000       1.500       2.000       2.000       2.000 <td>#</td> <td>(INCHES)</td> <td>(LBS)</td> <td>(INCHES)</td> <td>(LBS)</td> <td>(LBS/INCH)</td> <td>(LBS)</td> <td>(KSI)</td> <td>250</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)	250								
2       0.20       237       0.200       227       1.185       223       3.4         3       0.40       479       0.400       479       1.188       446       6.8         4       0.60       661       0.603       691       1.188       669       10.2         5       0.60       911       0.803       911       1.138       891       157         6       1.00       1145       1.037       1114       177       1       177       1.420       1385       1.133       1337       20.5         8       1.40       1577       1.125       1500       239       1       100       164       200       200       200       201       201       201       201       201       201       203       203       203       203       203       100       206       207       100       206       207       100       206       307       100       100       206       307       100       100       206       307       100       200       202       307       100       206       307       100       206       203       1.012       256       37.5       37.5       37.5	1	0.00	(== =)	0.000	0	N/A	0	(	2.54								
3       0.40       476       0.400       479       1.198       446       6.8         4       0.60       661       0.600       891       1.158       669       10.2         5       0.80       911       0.600       911       1.138       699       10.2         6       1.00       1.145       1.000       1.145       1.137       1114       17.1         7       1.20       1.385       1.133       1337       20.6       1       1       1         8       1.40       1.577       1.400       1577       1.122       1671       25.6       1	2	0.20	237	0.200	237	1,185	223	3.4									
4       0.60       661       0.600       891       1.158       669       102         5       0.600       911       0.800       911       1.138       891       13.7         6       1.00       1145       1.137       1114       17.7         7       1.20       1385       1.200       1385       1.137       1114       17.1         7       1.20       1385       1.137       1125       1560       23.9       9       150       1690       1.500       1690       1.122       1671       2.56         10       1.60       1.721       1.600       1.071       1894       2.600       7.7       1.22       1.770       1.894       2.600       7.7       1.224       1.003       1.071       1.694       2.600       7.7       7.7       2.00       2.022       1.032       2.451       3.7.5       7.7       7.200       2.200       2.200       2.200       1.032       2.451       3.7.5       7.7       7.200       2.200       2.200       2.200       2.200       3.9.3       9.3       5.00       0.000       0.600       1.000       1.500       2.000         18       20       200	3	0.40	479	0.400	479	1,198	446	6.8								~	-
5       0.80       911       0.800       911       1.138       891       13.7         6       100       1145       1.000       1145       1.137       1114       17.1         7       1.20       1365       1.200       1395       1.133       1337       20.5         8       1.40       1577       1.403       1577       1.122       1670       23.6         10       1.60       1781       1.114       1783       27.3       11       1.700       1880       1.107       1844       28.0         11       1.70       1.800       1.700       1.803       1.107       1844       28.0       30.7         12       1.86       1.970       1.803       2.117       32.4       35.8       37.5         16       2.20       2.000       2.200       1.032       2.451       37.5         18       -       -       -       -       -       -       -       -         20       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -       -	4	0.60	691	0.600	691	1,158	669	10.2	200								- <b>-</b>
6       100       1145       1.137       1114       17.1         7       120       1365       1.200       1385       1.137       1114       17.1         7       1.20       1365       1.200       1385       1.133       1337       20.5         8       1.40       1577       1.400       1577       1.122       1671       25.6         10       1.50       1660       1.500       1690       1.112       1671       25.6         11       1.70       1880       1.700       1890       1.107       1684       26.00         12       1.60       1970       1.990       2006       30.7       1       1.31       1.024       35.8         14       2.00       2.039       2.003       2.299       1.038       2.226       34.1         15       2.10       2.167       2.103       2.451       37.5       39.3       39.3         18       2.20       2.200       2.200       2.200       2.200       2.000       2.000       0.000       0.500       1.000       1.500       2.000         22       2       2       2       2       2       2       2	5	0.80	911	0.800	911	1,138	891	13.7	_	~							
7       120       1365       1.133       1337       20.5         8       1.40       1577       1.400       1577       1.125       1560       23.9         9       1.50       1690       1.500       1600       1.122       1671       25.6         10       1.600       1.781       1.102       1671       25.6         11       1.70       1.880       1.107       1.894       25.0         12       1.80       1.900       2.023       1.099       2.006       30.7         13       1.90       2.023       1.900       2.026       30.7         13       1.90       2.020       2.020       2.023       1.088       2.217         14       2.100       2.167       1.051       2.340       35.8         16       2.200       2.200       2.200       2.200       2.200       1.032         20       -       -       -       -       -       -         21       -       -       -       -       -       -         22       -       -       -       -       -       -       -         22       -       -	6	1.00	1145	1.000	1145	1,137	1114	17.1	(sc							A CONTRACTOR	
8       1.400       1577       1.400       1577       1.125       1560       23.9         9       1.500       1690       1.300       1990       1.122       1671       25.6         10       1.60       1781       1.600       1771       1.114       17783       27.3         11       1.70       1880       1.700       1890       1.107       1894       29.0         12       180       1970       1.800       1970       1.099       2006       30.7         13       190       2023       1.083       2117       32.4         14       2.00       2029       1.005       2229       34.1         15       2.10       2167       1.032       2451       37.6         16       2.20       2.200       2.200       2.203       39.3         18	7	1.20	1365	1.200	1365	1,133	1337	20.5	Ĩ							· · · · · · · · · · · · · · · · · · ·	
9       1 50       1690       1 500       1690       1 112       1671       25 6         10       1 60       1781       1.102       1671       25 6         10       1 60       1781       1.114       1783       27.3         11       1.170       1880       1.107       1894       26.0         12       1 80       1970       1.009       2006       30.7         13       1 90       2023       1.900       2023       3.083       2117       32.4         15       2 10       2 167       2.100       2 167       1.051       2 340       35.8         16       2 20       2 220       2 200       2 203       39.3       39.3         19       -       <	8	1.40	1577	1.400	1577	1,125	1560	23.9	0 15/	oo 🗆							
10       160       1781       1.114       1783       27.3         11       1.70       1880       1.107       1894       26.0         12       180       1970       1.800       1970       1.800       1970         13       190       2023       1.009       2006       30.7         14       2.00       2039       2.000       2099       1.068       2219         14       2.00       2.000       2.009       1.051       2.240       35.8         16       2.20       2.200       2.200       2.200       1.032       2.451       37.5         17       2.30       2.288       1.014       2.563       39.3       1.014       2.563       39.3         19	9	1.50	1690	1.500	1690	1,122	1671	25.6	8 15	~					/		
11       1.70       1880       1.700       1880       1.107       1894       29.0         12       180       1970       1800       1970       1.009       2006       30.7         13       190       2023       1.903       2017       32.4         14       2.00       2099       2.000       2099       1.088       2220       34.1         15       2.100       2167       2.100       2167       1.031       2340       35.8         16       2.202       2.200       2220       1.032       2451       37.5         17       2.30       2288       1.014       2563       39.3         18	10	1 60	1781	1.600	1781	1, <b>114</b>	1783	27.3	DAE								
12       180       1970       1.009       1.099       2006       30.7         13       190       2023       1.900       2023       1.083       2117       32.4         14       2.00       2099       2.000       2099       1.088       2212       34.1         15       2.10       2167       2.100       2167       1.051       2340       35.8         16       2.20       2.200       2.200       2.202       1.032       2.451       37.5         18	11	1.70	1880	1.700	1880	1,107	1894	29.0	2								
13       190       2023       1,000       2023       1,083       2117       32.4         14       2.000       2.000       2.000       2.000       2.000       2.000       2.000       3.5.8         16       2.20       2.220       2.200       2.220       1.032       2.451       37.5         17       2.30       2.288       1.014       2.563       39.3         19                20	12	1 80	1970	1 800	1970	1,099	2006	30.7	100					Τ			
14       2:09       2:099       2:099       1:088       2:228       34.1         16       2:10       2:167       2:100       2:17       1:051       2:340       35.8         17       2:30       2:288       2:300       2:283       1:014       2:563       39.3         18	13	1.90	2023	1.900	2023	1,083	2117	32.4	100	~						-	
15       210       2167       2.100       2167       1.051       2.240       35.8         16       220       2220       2.200       2220       1.032       2461       37.5         18       200       2288       2.300       2288       1.014       2563       39.3         18       200       200       2288       1.014       2563       39.3         19       200       200       200       200       200       200         20       200       200       200       200       200       200         21       200       200       200       2000       2000       2000       0.000       0.500       1.000       1.500       2.000         22       200       200       200       2.000       2.000       0.000       0.500       1.000       1.500       2.000         24       25       200       200       2.000       2.000       1.000       1.500       2.000         25       24       25       24       25       260       260       260       260       260       260       260       260       260       260       260       260       2.000<	14	2.00	2099	2.000	2099	1,068	2229	34.1							-		
16       2 20       2 220       2 220       2 220       1,032       2 451       37.5         17       2 30       2 288       2 300       2 288       1,014       2 563       39.3         19       19       100       1,500       2.000         20       1       1       1       1       10       100       1,500       2.000       1.000       1,500       2.000         21       1       1       1       1       1       1.000       1,500       2.000         22       1       1       1       1       1       1.000       1.000       1.500       2.000         23       1       1       1       1       1       1       1       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000       1.000 </td <td>15</td> <td>2.10</td> <td>2167</td> <td>2.100</td> <td>2167</td> <td>1,051</td> <td>2340</td> <td>35.8</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td>-</td> <td>_</td> <td></td> <td></td>	15	2.10	2167	2.100	2167	1,051	2340	35.8						-	_		
17       230       2288       2.300       2288       1,014       2563       39.3         18       19       100       1.500       2.000         20       1       1       1       10       100       1.500       2.000       1.000       1.500       2.000         21       1       1       1       10       1.500       2.000       1.000       1.500       2.000         22       1       1       1       1       1.500       2.000       1.500       2.000         23       1       1       1       1       1       1       1.500       2.000         24       1       1       1       1       1       1       1       1.500       2.000         25       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1       1	16	2.20	2220	2.200	2220	1,032	2451	37.5	51	<u></u>				-			
18  <	17	2.30	2288	2.300	2288	1,014	2563	39.3			/	*					
19       0	18																
20  <	19											*					
21 <td>20</td> <td></td>	20																
22	21									0 000		0.500	1	nan	15	00 2	000
23     Deflection (Incres)       24        25	22																
24     Collapse Line       25     Image: Collapse Line	73													DEFLECTION	(INCHES)		
	24											TE	ST LOAD-DEFI				
	25											- <b>-</b> - 12	or condident				

NOTES: 1. Positive load is down.

### Test Data

TEST #:		ELBOW -	F		TEST TYPE: COMBINED-45			
F=Fo-m		"m" TO BE B	ASED ON N DATA PO	NTS N =		9		
		THE VALUE	OF "m" =	1225	1225 Fo (LBS) = 50			
NOMINAL S	STRESS = M/Z KSI, M=F	×L.		L(IN) =	49.25	Z(IN <sup>3</sup> ) -	3 215	
ΠΑΤΑ	MEASURED		MÓDIEIED	)	SLOPE FOR	F	NÓMINAI	
POINT			DEFLECTION		START TO	BASED	STRESS	
1 0 11	BEIEEGIIOI	F	DEFECTION	F	DATA POINT	ON "m"	0111200	
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCE)	(LBS)	(KSI)	
1	2 30	2288	2.300	2288	N/A	2255	34.5	
2	2.10	1993	2.100	1993	1.475	2010	30.8	
3	1.90	1728	1.900	1728	1,400	1765	27.0	
4	1,70	1486	1.700	1486	1.336	1520	23.3	
5	1 50	1244	1 500	1244	1,298	1275	19.5	
6	1.30	1002	1.300	1002	1,274	1030	15.8	
7	1.10	767	1.100	767	1,255	785	12.0	
8	0.90	540	0.900	540	1,237	540	8.3	
9	0.70	305	0.700	305	1,225	295	4.5	
10	0.50	93	0.500	93	1,209	50	0.8	
11	0.41	0	0.410	0	1,198	60	0.9	
12								
13								
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FATIGUE - LOAD DEFLECTION CURVE

11/19/98

#### POSITIVE LOAD - UNLOADING CONDITION



NOTES:

## A-33

TEST #:		ELBOW - F			TEST TYPE	COMBINED-	45
F = Fo + m		"m" TO BE BASI	ED ON N DATA POINTS	N =		8	
		THE VALUE OF	"m" =	973		Fo (LBS) =	0
NOMINAL ST	RESS = M/Z KSI, M=F x	κ L.		L(IN) =	49.25	Z(IN <sup>3</sup> ) =	3.215
DATA	MEASURE	ED	MODIFIED	1	SLOPE FOR	F	NOMINAL
POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS
		F		F	DATA POINT	ON "m"	
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)
1	0.410	0	0.410	0	N/A	0	0.0
2	0.300	-87	0.300	-87	791	-107	-1.6
3	0.100	-307	0.100	-307	1,004	-302	-4.6
4	-0.100	-504	-0.100	-504	1,006	-496	-7.6
5	-0.300	-693	-0.300	-693	992	-691	-10.6
6	-0.500	-897	-0.500	-897	993	-886	-13.6
7	-0.700	-1079	-0.700	-1079	984	-1080	-16.6
8	-0.900	-1260	-0.900	-1260	973	-1275	-19.5
9	-1.100	-1442	-1.100	-1442	964	-14/0	-22.5
10	-1.300	-1616	-1.300	-1616	954	-1665	-25.5
11	-1.500	-1775	-1.500	-1775	941	-1859	-28.5
12	-1.600	-1856	-1.600	-1856	932	-1957	-30.0
13	-1.700	-1951	-1.700	-1951	927	-2054	-31.5
14							
15							
16							
17							
18							
19							
20							
21							
22							
23							
24							
25							



11/19/98

FATIGUE - LOAD DEFLECTION CURVE

NEGATIVE LOAD - LOADING CONDITION

NOTES:

TEST #:	#: ELBOW - F		TEST TYPE: COMBINED-45			FATIGUE - LOAD DEFLECTION CURVE							11/19/98		
F = Fo + m		'm" <sup>-</sup> O BE BAS THE VALUE OF	SED ON N DATA POINT = "m" =	S, N = 1118	1	2 Fo (LBS) =	-1200			NEGATIVE LOAD - UN	LOADING	CONDITION			
NOMINAL ST	RESS = M/Z KS , M=F x	L,		L(IN) = 4	19.25 Z	2( N <sup>3</sup> ) =	3.215								500
	MEASUDE		MODIEIED			C									500
POINT					STARLIO	BASED	SIRESS								
	DEFECTION	F	DEFECTION	F	DATA POINT	ON "m"	OTICEOU								
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)	-					-		0
1	-1 70	-1951	-1.700	-1951	N/A	-1200	-18.4	-2.5	00	-2.000	-1.9	-1	.000	-0.\$00	0.000
2	-1.73	-1866	-1.730	-1866	-2833	-1234	-18.9						and the second s		
3	-2.37	-1862	-2.370	-1862	-75	-1949	-29.9						<b>*</b> *		500
4	-2.13	-1597	-2.130	-1597	-207	-1681	-25.7								-500
5	-2.08	-1619	-2.080	-1619	-250	-1625	-24.9								
6	-2.00	-1506	-2.000	-1506	-249	-1536	-23.5	ŝ				A COLORING COLORING			
7	-1.80	-1279	-1.800	-1279	-5	-1312	-20.1	ĝ				/		_	-1000
8	-1.60	-1067	-1.600	-1067	405	-1088	-16.7	2							
9	-1.40	-855	-1.400	-855	/44	-864	-13.2	ă,							
10	-1.20	-620	-1.200	-620	954	-641	-9.8	Q							
11	-1.00	-401	-1.000	-401	1064	-41/	-6.4	õ							1500
12	-0.80	-181	-0.800	-181	1118	-193	-3.0	-							
13	-0.02	U	-0.620	0	114	8	0.1				•				
14											<b>`</b>				2000
16															
17															
18															
19								l l							2500
20												DEFLECTION (INCHES)			
21				· · · · · · · · · · · · · · · · · · ·								· , ····-,			
22															
23												TLOAD-DEFLECTION		ECTION	
24											* 120		- ENERG-DEN		
25															

NOTES: 1. At the beginning of the down bad, there was a problem with the software which required a shul down and restart. When restarted, the "offset" displacement was incorrect. The change in displacement and change in loads were correct.



### FATIGUE TEST DATA ANALYSIS

11/19/98

#### **ELBOW - F** TEST #:

## TYPE: COMBINED-45

AVERAGE STIFFNESS (lbs/in) = 1108 MOMENT ARM (in)= 49.25

D(III) = 4.5 $I(III) = 0.237$ $Z(IIII) = 0.0962(D - (D-21))(D - 3)$	D (in) = 4.5	t (in) = 0.237	Z (in <sup>3</sup> ) = 0.0982(D <sup>4</sup> -(D-2t) <sup>4</sup> )/D= 3.215
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TEST DISPLACEMENT/CYCLE DATA:

CONDITION	DISPLACEMENT	EFFECTIVE	NOMINAL	NUMBER
#	AMPLITUDE	APPLIED	STRESS	OF TEST
	(+/-) (in.)	LOAD (lbs)	(+/-) (psi)	CYCLES
	di		S	Ni
1	1.90	2105	32,238	2,917
2	0.00	0	0	0
3	0.00	0	0	0
4	0.00	0	0	0
5	0.00	0	0	0
6	0.00	0	0	0
7	0.00	0	0	0
8	0.00	0	0	0
			TOTAL CYCLES:	2,917

INCHES	DON δ = 1.9	EQUIVALENT NUMBER OF CYCLES, BAS
		IS: $N_{eq} = SUM(\delta_i / \delta_{max})^5 * N_i = 2,917$
1.541	$i = 245,000 * N_{eq}^{(-0.2)}/S =$	FOR NOMINAL DIMENSIONS:
i = 1.541		FOR Z(IN <sup>3</sup> ) = 3.215

COMMENTS:

L = 49.25 to the center of the elbow.
 Bolt replaced at 1245 cycles.

FP Form         the trade best de	TEST #:	t: ELBOW - G		TEST TYPE:	COMBINED	-45		F	ATIGUE - LOAD	DEFLECTION C	URVE			1	1/19/98			
NUMAL STRESS # VZ.KSI.MF¥L         L(H)         4 02         TH         2 12           DVIMAL STRESS # VZ.KSI.MF¥L         L(H)         4 02         X         Y         2 12           DVIMAL STRESS # VZ.KSI.MF¥L         L(H)         L(H)         KIP         2 12         2 12           DVIMAL STRESS # VZ.KSI.MF¥L         L(H)         L(H)         KIP         X         Y         2 12           DVIMAL STRESS # VZ.KSI.MF¥L         L(H)         DVIMAL STRESS # VZ.KSI.MF¥L         L(H)         X         Y         Y           DVIMAL STRESS # VZ.KSI.MF¥L         L(H)         DVIMAL STRESS # VZ.KSI.MF¥L         L(H)         X         Y         Y           DVIMAL STRESS # VZ.KSI.MF¥L         L(H)         DVIMAL STRESS # VZ.KSI.MF¥L         L(H)         X         Y	F = Fo + m		"m" TO BE E THE VALUE	BASED ON N DATA PO	NTS, N = 1102		10 Fo (LBS) -	0			POSITIVE	LOAD - LOADII	NG CONDITION					
Data         Medsure         MODIFIED         SLOPE FOR         F         NONINAL           POINT         DEFLECTION         LOAD         F         START TO         BASE         No         Street FOR         No         No           4         (INCHES)         (IBS)         (ILS)INCH)         (IBS)         (KS)         I         Image: Constraint of the street of the	NOMINAL S	TRESS = M/Z KSI, M=F	×L,		L( <b>IN</b> ) =	49.25	Z(IN <sup>3</sup> ) =	3.215										
DATA DPINT         MMESURE/ DEFLECTION         ICAD F         MODIFIES DATA PCINT         SLOPE FOR DATA PCINT         F         NONINAL BASED         STRESS           4         0.00         0         0.000         0         NAA         0           1         0.00         0         0.000         0         NAA         0           3         0.020         228         0.000         0         NAA         0           4         0.50         334         0.303         1115         1100         107           5         0.40         448         0.400         667         1112         661         0.61           6         0.50         561         0.500         667         1112         661         0.61           8         0.00         667         1105         81102         260         100           11         1.00         1061         1000         1061         1026         1122         681           12         1.10         1026         1021         1682         2620         2620           11         1.00         1061         1062         1102         681         1012         681         1012									3	00		1	1			1		
POINT         DEFLECTION         LOAD         STRAT TO F         BASE OF (LBS)         STRAT TO (LBS)         DSTRAT TO (	DATA	MEASURED		MODIFIED		SLOPE FOR	F	NOMINAL										
#         (INCHES)         F         DATA POINT (LBS)         ON Int (LBS)         (KS)           1         0.00         0         0.000         0         NNA         0           2         0.10         115         0.100         15         1.150         110         17           3         0.20         228         0.200         228         1.140         220         3.41           5         0.40         448         0.400         448         1.115         4.116         8.84           7         0.60         667         0.600         667         1.112         661         61           8         0.70         7.030         687         0.800         887         1.105         8.84           7         0.60         667         0.600         667         1.112         661         1.012         1.60           11         1.00         1.661         1.002         1.692         1.623         1.603         1.602         1.602         1.603           12         1.10         1.600         1.604         1.663         22.93         1.604         1.663         22.93         1.604         1.663         22.93         1.604 <td>POINT</td> <td>DEFLECTION</td> <td>LOAD</td> <td>DEFLECTION</td> <td>LOAD</td> <td>START TO</td> <td>BASED</td> <td>STRESS</td> <th></th> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS										
#         (INCHES)         (IBS)			F		F	DATA POINT	ON 'm"									_		
1         0.00         0         0.00         0         0.00         0           2         0.10         115         0.100         116         1110         17           3         0.20         228         0.200         228         1.140         220         3.4           4         0.30         334         0.300         3.34         1.115         5.1         0.40         4.48         0.400         4.48         0.400         4.48         0.400         4.48         1.115         6.61         0.61         6.67         0.800         6.67         1.112         6.61         0.61         6.7         0.800         6.67         1.100         6.681         7.3         1.060         1.012         6.81         7.3         1.000         1.091         1.026         1.922         7.52         1.111         1.000         1.091         1.026         1.022         7.62         1.01         1.991         1.026         1.022         7.63         7.70         1.000         1.991         1.026         1.322         2.03         1.401         1.402         1.402         1.402         1.402         1.402         1.402         1.402         1.666         1.532         2.23	#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)	2	00								
2         0.10         110         0.100         110 <td>- 1</td> <td>0.00</td> <td>115</td> <td>0.000</td> <td>115</td> <td>1 150</td> <td>110</td> <td>17</td> <th>-</th> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	- 1	0.00	115	0.000	115	1 150	110	17	-									
4       0.30       1.33       0.300       1.115       1.116       1.112       1.110       1.115       1.112       1.112       1.112       1.112       1.112       1.112       1.112       1.112       1.112	3	0.10	228	0.100	228	1.130	220	3.4	-						ر ر			
5       0.40       448       0.400       448       1.115       441       6.8         6       0.50       561       0.500       561       1.117       561       8.4         7       0.60       667       0.600       561       1.117       561       8.4         8       0.70       7/3       0.700       7/3       1.105       611       1.118         9       0.80       867       0.800       867       1.105       891       355         10       0.90       993       0.900       993       1.005       1102       69       150         11       1.00       1.000       1.001       1.005       1.102       69       150         13       1.20       1.266       1.200       1.995       1.024       2.203       1.01       1.000       1.011       1.076       1.432       2.19       1.00       1.001       1.902       1.024       2.83       2.037         16       1.50       1.700       1.704       1.603       2.63       2.77       3.36       3.77       1.656       1.873       2.837       2.00       3.04       2.00       3.04       0.00       0.500	4	0.20	334	0.300	334	1 115	331	51	1 .	~								
6       0.50       561       0.600       561       1117       561       84         7       0.60       667       0.600       667       1.112       661       0.1         8       0.70       7/3       0.00       667       1.105       881       7.35         9       0.60       867       0.800       887       1.105       881       7.35         10       0.90       993       0.900       993       1002       769       769         11       1.00       1061       1.000       1091       1065       1102       769         12       1.10       1190       1.067       1212       769       769       760         12       1.10       1190       1.067       1212       780       760       700         14       1.30       1401       1.300       1061       1783       2270       700         18       1.70       1795       1.700       1796       1.665       1673       2270         19       1.80       1871       1.002       1983       304       200       1.000       1.600       1.600       2.000       0.000       0.500	5	0.40	448	0.400	448	1,115	441	6.8	1 1									
7       0.60       667       0.600       667       1.112       661       10.1         8       0.70       7/3       0.700       7/3       0.700       7/3       0.700       7/3       0.700       7/3       0.700       7/3       0.700       7/3       0.700       7/3       0.700       7/3       0.700       7/3       0.700       7/3       0.700       7/3       1.105       881       1.31         10       0.90       993       0.900       993       1.102       992       52       52       51       50       100       100       1001       1001       1002       169       100	6	0.50	561	0.500	561	1.117	551	8.4	6									
8       0./0       //3       0.000       //3       1.105       //1       1.18         9       0.80       867       0.900       933       1.102       881       3.55         10       0.993       9.900       9.93       1.102       102       16.9         11       1.00       1091       1.000       1091       1.02       16.9         12       1.10       1190       1.100       1191       1.02       16.9         13       1.20       1.265       1.200       1295       1.081       1322       20.3         14       1.30       1401       1.300       1401       1.076       1432       21.9         15       1.40       1492       1.400       1492       1.064       165.3       27.0         18       1.70       1775       1.700       1785       1.056       1873       28.7         20       1.90       1962       1.900       1962       1.039       20.93       32.1         17       1.00       1962       1.900       1962       1.039       20.93       32.1         21       2.00       2.10       2.098       2.100       2.098	7	0.60	667	0.600	667	1.112	661	10.1	ļğ									
9       0.80       887       0.800       887       1.105       881       '3.5         10       0.90       993       0.900       993       1.102       992       '5.2         11       1.000       1091       1.025       992       '5.2         11       1.000       1091       1.025       992       '5.2         12       1.10       1190       1.100       1091       1.085       '102       '6.9         12       1.10       1190       1.026       1.202       1286       1.200       1296       1.081       '1322       20.3         14       1.30       1401       1.300       1401       1076       1432       21.9       '0.69       1542       23.6         16       1.50       1588       1.600       1774       1.661       1763       27.0       '0.66       1873       28.7         19       180       1871       1.900       1962       1.900       1962       1.039       2093       32.1         21       2.00       2.000       2.003       1.079       2703       33.8       30.4         22       2.10       2.008       2.100       2.009 <td>8</td> <td>0.70</td> <td>//3</td> <td>0.700</td> <td>1/3</td> <td>1.105</td> <td>(/1</td> <td>11.8</td> <th>] Ž 1</th> <td>00</td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td> <td></td>	8	0.70	//3	0.700	1/3	1.105	(/1	11.8	] Ž 1	00								
10       0.90       993       0.900       993       1.102       992       5.2         11       1.00       1091       1.005       1102       6.9         12       1.110       1190       1.005       1102       6.9         13       1.20       1286       1.200       1296       1.081       1322       20.3         14       1.30       1401       1.300       1401       1.076       1432       21.9         15       1.40       1492       1.400       1492       1.069       1542       23.6         16       1.50       1598       1.064       1653       25.3         17       1.60       1704       1.661       1763       27.0         18       1.70       1795       1.700       1795       1.656       1873       28.7         19       1.80       1871       1.647       1983       30.4       1.000       1.017       201.0       2098       1.017       21.10       200       20.00       20.00       20.00       20.00       20.00       20.00       2.000       1.000       1.500       2.000         22       2.10       2.002       2151       1.	9	0.80	887	0.800	887	1.105	881	13.5	l e									
11       1.00       1091       1.091       1.095       1102       1.69         12       1.10       1190       1.000       1190       1.087       1212       16.9         13       1.20       1296       1.200       1296       1.081       1322       20.3         14       1.30       1401       1.300       1401       1.076       1432       21.9         15       1.40       1492       1.000       1598       1.064       1542       23.6         17       1.60       1704       1.664       1763       27.0         18       1.70       1795       1.000       1962       1.039       2093       32.1         20       1.90       1962       1.039       2093       32.1       30.4         21       2.00       2000       2000       2003       1.017       2314       35.4         23       2.20       2151       2.000       2198       986       2534       38.8         25       1       1.040       2.993       38.6       1.040       1.050       2.000         24       2.30       2198       2.300       2198       986       2.534	10	0.90	993	0.900	993	1.102	992	15.2	4									
12       1.10       1190       1.000       1190       1007       1212       7.85         13       1.20       1296       1.200       1296       1.81       1322       20.3         14       1.30       1401       1.300       1401       1.076       1432       21.9         15       1.40       1492       1.400       1492       1.069       1542       23.6         16       1.50       1598       1.500       1598       1.064       1653       25.3         17       1.60       1704       1.601       1763       27.0         18       1.70       1795       1.056       1673       28.7         19       1.80       1871       1.047       1983       30.4         20       1.90       1962       1.039       2093       32.1         71       2.00       2030       1.079       2703       33.8         22       2.10       2098       2.107       2098       1.017       2314       354         23       2.20       2151       2.200       2161       1.002       2424       37.1         24       2.30       2198       2.300	11	1.00	1091	1.000	1091	1.095	1102	16.9	9									
13       1.20       1.29       1.20       1.40       1.20       1.40       1.20       1.40       <	12	1.10	1190	1.100	1190	1.087	1212	-8.6	1	00 -								
14       1.30       1401       1.300       1401       1.000       1432       21.3         15       1.40       1492       1.400       1492       1.069       1542       23.5         16       1.50       1598       1.000       1598       25.3         17       1.60       1704       1.601       1763       27.0         18       1.70       1795       1.000       1871       1.047       1983       30.4         20       1.90       1962       1.900       1962       1.039       2093       32.1         71       2.00       2.030       2.000       2.030       1.017       2314       35.4         22       2.101       2.008       2.100       2.098       1.017       2314       35.4         23       2.20       2151       2.000       2151       1.002       2424       37.1         24       2.30       2198       986       2534       38.8        •	14	1.20	1401	1.200	1401	1.001	1422	20.3	-						-			
10         17.0         158         17.00         158         1.000         164         1653         25.3           17         1.60         1704         1.600         1704         1.61         1763         27.0           18         1.70         1795         1.000         1871         1.047         1983         30.4           20         1.90         1962         1.900         19671         1.047         1983         30.4           20         1.90         1962         1.900         19671         1.047         1983         30.4           20         1.90         1962         1.900         19671         1.047         1983         30.4           21         2.00         2.030         2.098         1.017         2.314         35.4           23         2.20         2.151         2.000         2.151         1.002         2.424         37.1           24         2.30         2.198         2.300         2.198         9.86         2.534         38.8           25	14	1.30	1401	1.300	1401	1.070	1432	21.9	-			-						
17         1.60         1704         1.600         1704         1.061         1763         27.0           18         1.70         1795         1.700         1795         1.056         1873         28.7           19         180         1871         1.047         1983         30.4           20         1.90         1962         1.900         1962         1.039         2093         32.1           71         2.00         2030         2.000         2033         1.079         2703         33.8           22         2.10         2098         2.100         2098         1.017         2314         35.4           23         2.20         2151         2.200         2151         1.002         2424         37.1           24         2.30         2198         2.300         2198         986         2534         38.8           52             1.002         2424         37.1	16	1.50	1598	1.500	1598	1.064	1653	25.3	1									
18       1.70       1795       1.700       1795       1.056       1873       28.7         19       180       1871       1.800       1871       1.047       1983       30.4         20       1.90       1962       1.900       1962       1.039       2093       32.1         71       2.00       2.030       2.000       2.000       2.000       2.000       1.000       1.500       2.000         22       2.10       2.098       2.100       2.098       1.017       2.314       35.4         23       2.20       2151       2.200       2151       1.002       2.424       37.1         24       2.30       2198       2.300       2198       9.86       2.534       38.8         25	17	1.60	1704	1.600	1704	1.061	1763	27.0	1		× *							
19       180       1871       1 047       1983       30.4         20       1.90       1962       1.000       1962       1.039       2093       32.1         71       2.00       2030       2.000       2000       10.72       2703       33.8         22       2.10       2.098       2.100       2098       1.017       2314       35.4         23       2.20       2.151       2.200       2.151       1.002       2424       37.1         24       2.30       2.198       2.300       2.198       9.866       2.534       38.8         25	18	1.70	1795	1.700	1795	1.056	1873	28.7	1		× .							
20       1.90       1.962       1.000       1.962       1.039       2.993       3.21         21       2.00       2.030       2.000       2.030       1.029       2.203       3.3 8         22       2.21.01       2.0298       2.100       2.098       1.017       2.314       3.5 4         23       2.20       2.151       2.200       2.151       1.002       2.424       3.7 1         24       2.30       2.198       9.86       2.534       3.8 8       5.6 10.000       0.500       1.000       1.500       2.000         25	19	1.80	1871	1.800	1871	1.047	1983	30.4			1	T						
21         200         2030         2000         2030         1079         2703         33.8           22         2.10         2098         2.100         2098         1.017         2314         35.4           23         2.20         2151         2.200         2151         1.002         2424         37.1           24         2.30         2198         2.300         2.198         986         2534         38.8           25	20	1.90	1962	1.900	1962	1.039	2093	32.1										
22       2.10       2088       2.100       2098       1.017       2314       35.4         23       2.20       2151       2.200       2151       1.002       2424       37.1         24       2.30       2198       2.300       2198       986       2534       38.8         25 <th colspa<="" td=""><td>21</td><td>2 00</td><td>2030</td><td>2 000</td><td>2030</td><td>1.029</td><td>2203</td><td>33 8</td><th></th><td>0.000</td><td>0.</td><td>500</td><td>1.000</td><td>1.500</td><td>2.0</td><td>000</td><td>2.500</td></th>	<td>21</td> <td>2 00</td> <td>2030</td> <td>2 000</td> <td>2030</td> <td>1.029</td> <td>2203</td> <td>33 8</td> <th></th> <td>0.000</td> <td>0.</td> <td>500</td> <td>1.000</td> <td>1.500</td> <td>2.0</td> <td>000</td> <td>2.500</td>	21	2 00	2030	2 000	2030	1.029	2203	33 8		0.000	0.	500	1.000	1.500	2.0	000	2.500
23         2.20         2151         2.200         2151         1.002         2424         37.1           24         2.30         2198         2.300         2198         2534         38.8           25	22	2.10	2098	2.100	2098	1.017	2314	35.4	-		•		DEEL ECTION				2.000	
24         2.30         2198         2.300         2198         986         2534         38.8           25	23	2.20	2151	2.200	2151	1.002	2424	37.1	4				DEFLECTION	(INCHES)				
	24	2.30	2198	2.300	2198	986	2534	38.8	4					NEAR-DEFLECTION		SELINE		
	25								J			+ TEOTEONDOL			- OOLLAI	- =//1L		

NOTES: 1. Positive load is down.

2.000

11/19/98

2.500

TEST #:	'EST #: ELBOW - G			TEST TYPE:	TEST TYPE: COMBINED-45			FA	FATIGUE - LOAD DEFLECTION CURVE	
F = Fo + m		"m" TO BE BA	ASED ON N DATA POI OF "m" =	NTS, N = 1121		12 Fo (LBS) =	200			POSITIVE LOAD - UNLOADING CONDITION
NÓMINAL S	IRESS = M/Z KSI, M=H	×L,		L(IN) =	49.25	Z(IN <sup>3</sup> ) =	3 215			
DATA POINT #	MEASURED DEFLECTION (INCHES)	) F (LBS)	MODIFIED DEFLECTION (INCHES)	) F (LBS)	S_OPE FOR START TO DATA POINT (LBS/INCH)	F BASED ON "m" (LBS)	NOMINAL STRESS (KSI)		2500	
2 3 4	2.10 2.00 1.80	1939 1825 1591	2.000 2.100 2.000 1.800 4.000	1939 1825 1591	1,295 1,251 1,211	1882 1770 1546	28.8 27.1 23.7		2000	
6 7 8 9	1.00 1.40 1.20 1.00 0.80	1137 910 682 463	1.000 1.400 1.200 1.000 0.800	11349 1137 910 682 463	1,64 1,18 1,164 1,157 1,149	1097 873 649 424	202 16.8 13.4 9.9 6.5	(Sanuc	1500	
10 11 12 13	0.60 0.40 0.34	251 39 0	0.600 0.400 0.340	251 39 0	1,140 1,131 1,121	200 -24 -92	3.1 -0.4 -1.4	LOAD (F	1000	
14 15 16 17									500	
18 19 20 21 22									0 0.0	0 0.q00 0.500 1.q00 1.500 2.q0
23 24 25									-500	D DEFLECTION (INCHES)
I			1							-+ TEST LOAD-DEFLECTION -= LINEAR-DEFLECTION

NOTES:

TEST #:		ELBOW - G	;		TEST TYPE:	COMBINED-	45
F – Fo + m		"m" TO BE BAS		N =		в	
		THE VALUE OF	= 'm' =	964		Fo (LBS) =	0
NOMINAL ST	RESS = M/Z KSI, M=F >	۲L,		L(IN) =	49.25	Z(IN <sup>3</sup> ) =	3.215
	MEASURE	-n	MODIFIER	1	SLOPE FOR	F	NOMINAL
POINT	DEELECTION		DEFLECTION		START TO	BASED	STRESS
	SET LEG HOIT	F	02122011011	F	DATA POINT	ON "m"	0111200
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)
1	0.34	0	0.34	0	N/A	) ) )	0.0
2	0.20	-141	0.20	-141	1,007	-135	-2.1
3	0.00	-346	0.00	-346	1,018	-328	-5 0
4	-0.20	-527	-0.20	-527	980	-521	-8.0
5	-0.40	-724	-0.40	-724	975	-713	-10.9
6	-0.60	-921	-0.60	-921	975	-906	-13.9
7	-0.80	-1118	-0.80	-1118	976	-1099	-16.8
8	-1.00	-1284	-1.00	-1284	964	-1292	-19.8
9	-1.10	-1368	-1.10	-1368	955	-1388	-21.3
10	-1.20	-1451	-1.20	-1451	946	-1485	-22.7
11	-1.30	-1557	-1.30	-1557	944	-1581	-24.2
12	-1.40	-1617	-1.40	-1617	936	-1677	-25.7
13	-1.50	-1706	-1.50	-1706	929	-1774	-27.2
14	-1.60	-1776	-1.60	-1776	921	-1870	-28.6
15	-1.70	-1844	-1.70	-1844	912	-1967	-30.1
16							
17							
18							
19							
20							
21							
22							
23							

#### FATIGUE - LOAD DEFLECTION CURVE

#### 11/19/98

0.500

#### NEGATIVE LOAD - LOADING CONDITION



25 NOTES:

24



NOTES: 1. At the beginning of the download, there was a problem with the software which required a shut down and restart. When restarted, the "offset" displacement was incorrect. The change in displacement and change in loads were correct.

LOAD (POUNDS) -1.5 .5 A states DEFLECTION (INCHES) 

SUMMARY LOAD-DEFLECTION

### FATIGUE TEST DATA ANALYSIS

11/19/98

## TEST #: ELBOW - G

## TYPE: COMBINED-45

AVERAGE STIFFNESS (lbs/in) = 1054 MOMENT ARM (in)= 49.25

D (in) = 4.5 t (in) = 0.237 Z (in<sup>3</sup>) = 0.

 $Z (in^3) = 0.0982(D^4-(D-2t)^4)/D= 3.215$ 

TEST DISPLACEMENT/CYCLE DATA:

CONDITION	DISPLACEMENT	EFFECTIVE	NOMINAL	NUMBER
#	AMPLITUDE	APPLIED	STRESS	OF TEST
	(+/-) (in.)	LOAD (lbs)	(+/-) (psi)	CYCLES
	di		S	Ni
1	2.03	2135	32,703	2,157
2	0.00	0	0	0
3	0.00	0	0	0
4	0.00	0	0	0
5	0.00	0	0	0
6	0.00	0	0	0
7	0.00	0	0	0
8	0.00	0	0	0
			TOTAL CYCLES:	2,157

DON $\delta$ = 2.025	INCHES
i = 245,000 * N $_{eq}^{(-0.2)}/S$ =	1.614
	i = <b>1.614</b>
	$\delta = 2.025$ i = 245,000 * N <sub>eq</sub> <sup>(-0.2)</sup> /S =

COMMENTS:

TEST #:	ELBOW - H			TEST TYPE: COMBINED-45			FATIGUE - LOAD DEFLECTION CURVE					11/19/9	8				
F = Fo + m		"m" TO BE BASED ON N DATA POINTS, N = THE VALUE OF "m" = 108!		NNTS, N = 1089	10 Fo (LBS) = 0		POSITIVE LOAD - LOADING CONDITION										
NOMINAL S	TRESS = M/Z KSI, M=F	∓xL,		L(IN) =	49.25	Z(IN <sup>3</sup> ) =	3.215										
	NEAGURE	<u> </u>	NODIFIER				NOMINAL	3	3000								٦
	DEFLECTION	1000			SLOPE FOR		NUMINAL										
POINT	DEFLECTION	LUAD	DEFLECTION	LOAD		DASED	SIRESS										
#	(INCHES)	(189)	(INCHES)	(LBS)			(KSI)										
1	(1101120)	(100)	0.000	(100)	(CDO/INOIT) N/A	(105)	(((0))	2	2500	1		1	t				1
2	0.00	104	0 100	104	1 040	109	17										
3	0.30	332	0.300	332	1 111	327	5.0										
4	0.50	551	0.500	551	1,108	545	8.3	_						K			
5	0.70	770	0.700	770	1,104	762	11.7	2	2000 -					X			1
6	0.90	990	0.900	990	1,103	980	15.0							×			
7	1.10	1210	1.100	1210	1,102	1198	18.3	æ									
8	1.30	1414	1.300	1414	1,094	1416	21.7	ã.									
9	1.40	1527	1.400	1527	1,092	1525	23.4	- <u>5</u> 1	1500								1
10	1.50	1628	1.500	1628	1,089	1634	25.0	2									
11	1.60	1724	1.600	1724	1,084	1742	26.7	ğ				- I A					
12	1.70	1822	1.700	1822	1,079	1851	28.4	<u>ه</u>						-			
13	1 80	1921	1.800	1921	1,074	1960	30.0	- 1	1000					-			1
14	1.90	2019	1.900	2019	1,069	2069	31.7										
15	2.00	2095	2.000	2095	1,061	2178	33.4										
16	2.10	2140	2.100	2140	1,947	2287	35.0				/						
17	2.20	2220	2.200	2220	1,034	2396	36.7		500								1
10	2.30	2290	2.300	2290	1,020	2000	38.4				-						
20	2.40	2330	2.400	2330	1,003	2014	40.0				•						
20										<u>x</u>							
22									0.00		00	4 000	4 500			2 500 2	
23									0.00	0 0.5	00	1.000	1.500	2		2.500 3.	000
24													DEFLECTION (I	NCHES)			
26											TE	ST LOAD	DEFLECTION	- LINEAR-DEFLI	ECTIONCOL	LAPSE LINE	

25 NOTES:

NOTES: 1. Positive oad is down.

A-45

11/19/98

FATIGUE - LOAD DEFLECTION CURVE

POSITIVE LOAD - UNLOADING CONDITION

TEST #:	ELBOW - H		TEST TYPE	COMBINE	D-45
F = Fo + m).	"m" TO BE BASED ON N DATA PC THE VALUE OF "m" =	NTS, N - 1184		11 Fo (LBS) =	= 200
NOM NAL STRESS = M/Z KSI, M=	FxL,	L(IN) =	49.25	Z(IN <sup>3</sup> ) =	3.215

DATA	MEASURED		MODIFIED		SLOPE FOR	- F	NOMINAL
POINT	DEFLECTION	LOAD	LOAD DEFLECTION LOAD START TO		START TO	BASED	STRESS
		F		F	DATA POINT	ON "m"	
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)
1	2.40	2330	2.400	2330	N/A	2331	35.7
2	2.20	2030	2.200	2030	1,500	2094	32.1
3	2.00	1985	2.000	1985	862	1857	28.4
4	1.80	1550	1.800	1550	1,192	1621	24.8
5	1.60	1308	1.600	1308	1,262	1384	21.2
6	1.40	1066	1.400	1066	1,274	ŕ 147	17.6
7	1.20	839	1.200	839	1,264	910	13.9
8	1.00	619	1.000	619	1,246	6/4	10.3
9	0.80	400	0.800	400	1,227	437	6.7
10	0.60	203	0.600	203	1,205	200	3.1
11	0.40	0	0.400	0	1,184	-37	-0.6
12							
13							
14							
15							
16							
17							
18							
19							
20							
21							
22							
23							
24							
25							



NOTES:

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12

13 14

15

16

17

18 19 20

21 22

23 24

25

-0 60

-0 80

1 00

-1 20

-1 40

-1 50

-1 60

-1.70

-1.80

-1.90

-2.00

-2.10

-931

-1106

1280

-1461

-1620

-1688

-1787

-1855

-1923

-2006

-2089

-2165

TEST #:		ELBOW - H	I		TEST TYPE: COMBINED-45			
F = Fo + m: :		"m" TO BE BAS THE VALUE OF	SED ON N DATA POINTS - "m' =	, N - 915		8 Fo (LBS) = 0		
NOMINAL S	TRESS = M/Z KSI, M=F >	κL,		L(IN) - 49.25			3.215	
DATA	MEASURE	ED	MODIFIED	)	SLOPE FOR	F	NOMINAL	
POINT	DEFLECTION	LOAD	DEFLECTION	LOAD	START TO	BASED	STRESS	
#	(INCHES)	(LBS)	(INCHES)	(LBS)	(LBS/INCH)	(LBS)	(KSI)	
1	0.40	0	0.40	0	N/A	0	0.0	
2	0.20	-190	0.20	-190	950	-183	-2.8	
3	0.00	-387	0.00	-387	968	-366	-5.6	
4	-0.20	-561	-0.20	-561	940	-549	-8.4	
5	-0.40	-757	-0.40	-757	943	-732	-11.2	

-0.60

-0.80 1.00

-1.20

-1.40

-1 50

-1.60

-1.70

-1.80

-1.90

-2.00

-2.10

-931

-1106

1280

-1461

-1620

-1688

-1787

-1855

-1923

-2006

-2089

-2165

933

923

915

910

902

892

889

883

876

871

867

862

-915

-1098

1281

-1464

-1647

-1738

-1830

-1921

-2012

-2104

-2195

-2287

-14.0

-16.8 19.6

-22.4

-25.2 -26.6

-28.0

-29.4

-30.8

-32.2

-33.6

-35.0

#### FATIGUE - LOAD DEFLECTION CURVE

#### NEGATIVE LOAD - LOADING CONDITION



######

NOTES:



NOTES

1. At the beginning of the download, there was a problem with the software which required a shut down and restart. When restarted, the "offset" displacement was incorrect. The change in displacement and change in loads were correct.



## FATIGUE TEST DATA ANALYSIS

11/19/98

## TEST #: ELBOW - H

# TYPE: COMBINED-45

AVERAGE STIFFNESS (lbs/in) = 1057 MOMENT ARM (in)= 49.25

D (in) = 4.5 t (in) = 0.237 Z (i

 $Z(in^3) = 0.0982(D^4-(D-2t)^4)/D= 3.215$ 

TEST DISPLACEMENT/CYCLE DATA:

CONDITION	DISPLACEMENT	EFFECTIVE	NOMINAL	NUMBER
#	AMPLITUDE	APPLIED	STRESS	OF TEST
	(+/-) (in.)	LOAD (lbs)	(+/-) (psi)	CYCLES
	di		S	Ni
1	2.02	2135	32,708	2,230
2	2.08	2194	33,599	490
3	0.00	0	0	0
4	0.00	0	0	0
5	0.00	0	0	0
6	0.00	0	0	0
7	0.00	0	0	0
8	0.00	0	0	0
			TOTAL CYCLES:	2,720

EQUIVALENT NUMBER OF CYCLES, BASE	$ED ON \delta = 2.075$	INCHES
IS: $N_{eq} = SUM(\delta_i / \delta_{max})^5 * N_i = 2,440$		
FOR NOMINAL DIMENSIONS:	i = 245,000 * N $_{eq}^{(-0.2)}/S$ =	1.532
FOR Z(IN <sup>3</sup> ) = 3.215		i = 1.533

COMMENTS: